Western Technologies
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GEOTECHNICAL EVALUATION REPORT

THE VILLAGE AT SADDLEROCK CROSSING

APN #: 408-26-004B&C, 009A&C, 010-014, 086A, 088
1259 West State Route 89A
Sedona, Arizona
WT Job No. 2523JB119

PREPARED FOR:

Mr. Curt Baney
475 Northeast Bellevue Drive, Suite 210
Bend, Oregon 97701
Attn: Mr. Art Beckwith, P.E.

August 25, 2023

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GEOTECHNICAL ENVIRONMENTAL INSPECTIONS NDT MATERIALS

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*Building **Confidence** from the Ground Up*

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August 25, 2023

Mr. Curt Baney
475 Northeast Bellevue Drive, Suite 210
Bend, Oregon 97701

Attn: Mr. Art Beckwith, P.E.

Re: Geotechnical Evaluation — Job No. 2523JB119
The Village at Saddlerock Crossing
APN #: 408-26-004B&C, 009A&C, 010-014, 086A, 088
1259 West State Route 89A
Sedona, Arizona

Western Technologies Inc. has completed the geotechnical evaluation for the resort to be located in Sedona, Arizona. This study was performed in general accordance with our proposal number 2523PB218 dated June 9, 2023. The results of our study, including the boring location diagram, laboratory test results, boring logs, and the geotechnical recommendations are attached.

We have appreciated being of service to you in the geotechnical engineering phase of this project and are prepared to assist you during the construction phases as well. If design conditions change, or if you have any questions concerning this report or any of our testing, inspection, design and consulting services, please do not hesitate to contact us. We look forward to working with you on future projects.

Sincerely,
WESTERN TECHNOLOGIES, INC.
Geotechnical Engineering Services

Gregory L. E. Burr, R.G., E.I.T.
Geotechnical Department Manager

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1259 WEST STATE ROUTE 89A
SEDONA, ARIZONA
JOB NO. 2523JB119

1.0 PURPOSE

This report contains the results of our geotechnical evaluation for the proposed resort to be located at 1259 West State Route 89A, Arizona. The purpose of these services is to provide information and recommendations regarding:

- Subsurface conditions
- Foundation design parameters
- Slabs-on-grade
- Lateral earth pressures
- Seismic considerations
- Earthwork guidelines
- On-site pavements
- Drainage
- Corrosivity (soil to concrete)

Results of the field exploration, field tests, and laboratory testing program are presented in the Appendices.

2.0 PROJECT DESCRIPTION

Based on information provided by Mr. Art Beckwith, P.E., the proposed project will consist of nine two-story multi-family dwellings, four two-story hotel buildings, one single-story lobby/restaurant building, and one two-story parking garage with one below-grade lower level. Assumed plan areas range between 1,500 and 10,000 square feet to be constructed on eleven lots with a total combined area of 6.29-acres. The building structures are assumed to be wood frame and/or masonry construction with slab-on-grade floors, and the parking garage will be concrete construction with a slab-on-grade bottom floor. Maximum wall and column loads for the structures are assumed to be 4 kips per linear foot and 80 kips, respectively. We anticipate no extraordinary slab-on-grade criteria, and that the finished building first floor levels will be within 2 to 4 feet of the existing site grades and that the parking garage will be a maximum of 15 feet below the existing site grades. In addition, asphalt paved parking and drive areas will be included as part of the proposed development. Should any of our information or assumptions not be correct, we request that the Client notify Western Technologies (WT) immediately.

3.0 SCOPE OF SERVICES

3.1 Field Exploration

Nineteen borings were auger drilled to depths of about 4 to 21 feet below existing site grades in the proposed building and pavement areas. The borings were at the approximate locations shown on the attached Boring Location Diagram. A field log was prepared for each boring. These logs contain visual classifications of the materials encountered during drilling as well as interpolation of the subsurface conditions between samples. Final logs, included in Appendix A, represent our interpretation of the field logs and may include modifications based on laboratory observations and tests of the field samples. The final logs describe the materials encountered, their thickness, and the locations where samples were obtained.

The Unified Soil Classification System was used to classify soils. The soil classification symbols appear on the test pit logs and are briefly described in Appendix A. Local and regional geologic characteristics were used to estimate the seismic design criteria and liquefaction potential.

3.2 Laboratory Analyses

Laboratory analyses were performed on representative soil samples to aid in material classification and to estimate pertinent engineering properties of the on-site soils for preparation of this report. Testing was performed in general accordance with applicable standard test methods. The following tests were performed and the results are presented in Appendix B.

- Water content
- Dry density
- Compression
- Maximum density/optimum moisture
- Remolded expansion

- Gradation
- Plasticity
- Soluble salt/sulfate/chloride content

Test results were utilized in the development of the recommendations contained in this report.

3.3 Analyses and Report

This geotechnical engineering report includes a description of the project, a discussion of the field and laboratory testing programs, a discussion of the subsurface conditions, and design recommendations as appropriate to the purpose. The scope of services for this project does not include, either specifically or by implication, any environmental assessment of the site, discovery of underground storage tanks or other underground structures, or identification of contaminated or hazardous materials or conditions. If there is concern about the potential for such contamination, other studies should be undertaken. We are available to discuss the scope of such studies with you.

4.0 SITE CONDITIONS

4.1 Surface

At the time of our exploration, the site was eleven undeveloped commercial lots. The site was bordered on the north by West State Route 89A, on the south by a residential subdivision, and on the east and west by developed commercial lots. The native ground surface was smooth and exhibited a gentle slope slope down to the west. Dispersed stockpiles of native soils and large sandstone boulders were present in some areas across the site. Site surface drainage appeared to be fair to poor by means of sheet flow to the west. Evidence of previous surface water ponding was observed in some portions of the site at the time of our field exploration. Vegetation on the site consisted of a sparse growth of native trees, bushes, cacti, grasses and weeds.

4.2 Subsurface

As presented on the Boring Logs, surface and subsoils extending to the full depth of exploration in all of the locations explored included low plasticity, firm to hard Sandy, Silty

CLAYS with variable amounts of gravel; low plasticity, stiff to hard Sandy CLAYS with variable amounts of gravel; low plasticity Silty, Clayey GRAVELS with variable amounts of sand; and non-plastic, soft SILTS with variable amounts of gravel. Refusal to auger penetration occurred in about three-quarters of the borings at depths of about 4 to 17 feet below the existing site grades on SANDSTONE. Groundwater was not encountered in any boring at the time of exploration. A detailed description of the soils encountered can be found on the boring logs in Appendix A.

The boring logs included in this report are indicators of subsurface conditions only at the specific location and date noted. Variations from the field conditions represented by the borings may become evident during construction. If variations appear, we should be contacted to re-evaluate our recommendations.

5.0 GEOTECHNICAL PROPERTIES AND ANALYSIS

5.1 Laboratory Tests

Laboratory test results (see Appendix B) indicate that on-site subsoils located near and below anticipated foundation levels exhibit low to moderate compressibility at existing water contents. Moderate to high additional compression occurs when the water content is increased.

Near-surface soils are of nil to low plasticity. These soils exhibit low expansion potential when recompacted, confined by loads approximating floor loads and saturated. Slabs-on-grade supported on recompacted on-site soils have a low potential for heaving if the water content of the soil increases.

5.2 Field Tests

On-site subsoils located near and below anticipated foundation levels exhibited low to high resistance to penetration using the ring-lined barrel sampler (ASTM D3550). The penetration resistance values also exhibited some variability between test locations and with depth. This represents a potential for differential movements within structures supported on existing soils in their present condition.

6.0 RECOMMENDATIONS

6.1 General

Recommendations contained in this report are based on our understanding of the project criteria described in Section 2.0 and the assumption that the soil and subsurface conditions are those disclosed by the explorations. Others may change the plans, final elevations, number and type of structures, foundation loads, and floor levels during design or construction. Substantially different subsurface conditions from those described herein may be encountered or become known. Any changes in the project criteria or subsurface conditions shall be brought to our attention in writing.

6.2 Design Considerations

Proposed finished floor elevations and the site grading scheme may result in different foundation bearing materials/conditions across the building areas (native soil/subbase fill and dense rock). In order to minimize possible differential foundation movements, all structure foundation elements should bear either entirely on engineered fill or entirely on dense rock.

Laboratory test results indicate that the site soils become weaker and more compressible with an increase in moisture content under typical foundation loadings. These soils are not considered suitable for support of foundations in their present state and should be over-excavated and recompacted as recommended in the **EARTHWORK** section of this report. Proper drainage should be provided to help prevent infiltration of moisture below the foundations and concrete slabs.

Cobbles and some boulders may be encountered. Oversized materials, greater than 3 inches, could present construction difficulties for foundation, utility trenches and other excavations. In cut areas and excavations, exposed oversized materials should be removed.

6.3 Conventional Spread Foundations

If all foundation elements are deepened as necessary to bear on dense rock, as in the case of the basement level for the parking garage, footings may be designed to impose a maximum dead plus live-load pressure of up to 4000 pounds per square foot. If desired, lean mix (2-sack) concrete backfill may be used between the design bottom of footing elevation and the top of the dense rock. Settlement of foundation elements bearing on dense rock or on lean mix concrete backfill extending to dense rock should be nominal.

If all foundation elements are designed to bear on engineered fill, the proposed structures can be supported by conventional shallow spread footings bearing on a minimum thickness of 1.5 feet of low expansive on-site soils removed and recompacted as engineered fill and/or properly compacted, imported, low expansive, engineered fill. Alternative footing depths and design bearing capacities are presented in the following tabulation:

Footing Depth Below Finished Grade[1] (ft)	Allowable Bearing Capacity[2] (psf)
1.5	2000
2.0[3]	2500

We anticipate that total settlement of the proposed structures, supported as recommended on engineered fill, should be less than 1 inch. Differential settlement should be less than ¾ inch. Additional foundation movements could occur if water from any source infiltrates the foundation soils.

The design bearing capacities apply to dead loads plus design live load conditions. Recommended minimum widths of column and wall footings are 24 inches and 16 inches, respectively. The bearing values given are net bearing values and the weight of the concrete in the footings may be ignored.

All footings, stem walls and masonry walls should be reinforced to reduce the potential for distress caused by differential foundation movements. The use of joints at openings or other discontinuities in masonry walls is recommended.

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[1] Finished grade is the lowest adjacent grade for perimeter footings and floor level for interior footings.

[2] Allowable bearing capacities assume fulfillment of **EARTHWORK** recommendations. Pounds per square foot (psf).

[3] Minimum perimeter footing depth based on anticipated frost penetration and recommended bearing capacity.

Site preparation procedures and foundation excavations should be observed by the geotechnical engineer to assess that adequate bearing conditions exist and that recompaction of native soils and/or placement of engineered fill has been performed satisfactorily. If the soil conditions encountered differ significantly from those presented in this report, supplemental recommendations will be required.

6.4 Lateral Design Criteria

For retaining walls located above any free water surface with no surcharge loads, recommended equivalent fluid pressures and coefficients of base friction for unrestrained elements are:

- Active:
  Undisturbed subsoil .... 36 psf/ft
  Compacted granular backfill .... 30 psf/ft
  Compacted site soils .... 36 psf/ft

- Passive:
  Shallow wall footings .... 250 psf/ft
  Shallow column footings .... 375 psf/ft

- Coefficient of base friction 0.35*

  * The coefficient of base friction should be reduced to 0.25 when used in conjunction with passive pressure.

Where the design includes restrained elements, the following equivalent fluid pressures are recommended:

- At-rest:
  Undisturbed subsoil .... 60 psf/ft
  Compacted granular backfill .... 55 psf/ft

These lateral earth pressures are not applicable for submerged soils. We should be consulted for additional recommendations if such conditions are to be included in the design. Any surcharge from adjacent loadings must also be considered. Walls below grade should be waterproofed.

We recommend a free-draining soil layer or manufactured geocomposite material, be constructed adjacent to the back of the retaining wall. A filter may be required between the soil backfill and drainage layer. This drainage zone should help prevent hydrostatic pressure buildup. This vertical drain should be tied into a gravity drainage system at the base of the retaining wall. It is important that all backfill be properly placed and compacted. Backfill should be mechanically compacted in layers. Flooding or jetting should not be permitted. Care should be taken not to damage the walls when placing the backfill. Backfills should be inspected and tested during placement.

Fill against footings, stem walls and retaining walls should be compacted to densities specified in **EARTHWORK**. Medium to high plasticity clay soils should not be used as backfill against retaining walls. Compaction of each lift adjacent to walls should be accomplished with hand-operated tampers or other lightweight compactors. Overcompaction may cause excessive lateral earth pressures which could result in wall movements.

6.5 Seismic Considerations

Structures should be designed in accordance with applicable building codes. The seismic design parameters presented in the following table, in accordance with the 2018 International Building Code and ASCE 7-16, are applicable to the project site:

Seismic Design Parameters International Building Code 2018, ASCE 7-16	
Soil Site Class	C
Mapped Spectral Response Acceleration at 0.2 sec period (S_s)	0.294g
Mapped Spectral Response Acceleration at 1.0 sec period (S_1)	0.093g
Site Coefficient for 0.2 sec period (F_a)	1.3
Site Coefficient for 1.0 sec period (F_v)	1.5
Design Spectral Response Acceleration at 0.2 sec period (S_{DS})	0.255g
Design Spectral Response Acceleration at 1.0 sec period (S_{D1})	0.093g

The soil site class is based upon conditions identified in shallow exploratory borings and local knowledge of the geotechnical conditions in the vicinity of the site. Conditions extending beyond the depth of our borings to a depth of 100 feet were assumed for the purposes of providing the information presented in the table. Based upon the density of the on-site soils, the relatively shallow rock conditions and lack of groundwater, the potential settlement and lateral spread due to liquefaction is not considered to be a significant concern on this site.

6.6 Conventional Slab-on-Grade Support

Floor slabs can be supported on properly placed and compacted fill or approved, properly recompacted native soils. For design of interior slabs-on-grade, we recommend using a modulus of subgrade reaction (k) of 250 pounds per cubic inch (pci) for the on-site soils or imported fill material, based on a 30-inch diameter plate. The slab subgrade should be prepared by the procedures outlined in this report. A minimum 4-inch thick layer of base course should be provided beneath all slabs to help prevent capillary rise and a damp slab. The use of vapor retarders is desirable for any slab-on-grade where the floor will be covered by products using water-based adhesives, wood, vinyl backed carpet, impermeable floor coatings (urethane, epoxy, acrylic terrazzo, etc.) or where the floor will be in contact with moisture sensitive equipment or product. When used, the design and installation should be in accordance with the guidance provided in ACI 302.1R and 302.2R. Final determination on the use of a vapor retarder should be left to the slab designer.

All concrete placement and curing operations should follow the American Concrete Institute manual recommendations. Improper curing techniques and/or high slump (water-cement ratio) could cause excessive shrinkage, cracking or curling. The plastic properties of the concrete should be documented at the time of placement and specimens should also be prepared for strength testing to verify compliance with project specifications. Concrete slabs should be allowed to cure adequately before placing vinyl or other moisture sensitive floor covering.

6.7 Drainage

The major cause of soil-related foundation and slab-on-ground problems is moisture increase in soils below structures. Properly functioning conventional foundations and floor slabs-on-ground require appropriately constructed and maintained site drainage conditions. Therefore, it is extremely important that positive drainage be provided during construction and maintained throughout the life of the structures. It is also important that proper planning and control of landscape and irrigation practices be performed.

Infiltration of water into utility or foundation excavations must be prevented during construction. Backfill against footings, exterior walls, and in utility and sprinkler line trenches should be well compacted and free of all construction debris to minimize the possibility of moisture infiltration. If utility line trenches are backfilled with a granular material, then a clay or concrete plug should be placed in the trench adjacent to the structures to prevent water from following the trench back under the structures.

In areas where sidewalks, patios or driveways do not immediately adjoin the structures, protective slopes should be provided with an outfall of about 5 percent for at least 10 feet from perimeter walls. Scuppers and/or gutters and drain pipes should be designed to provide drainage away from the structures for a minimum distance of 10 feet. Planters or other surface features that could retain water adjacent to the structures should be avoided if at all possible. If planters and/or landscaping are adjacent to or near the structures, there will be a greater potential for moisture infiltration, soil movement and structure distress. As a minimum, we recommend the following:

- Grades should slope away from the structures.
- Planters should slope away from the structures and should not pond water. Drains should be installed in enclosed planters to facilitate flow out of the planters.
- Only shallow rooted landscaping should be used.
- Watering should be kept to a minimum. Irrigation systems should be situated on the far side of any planting and away from the structures to minimize infiltration beneath foundations from possible leaks.
- Trees should be planted no closer than a distance equal to three-quarters of their mature height or 15 feet, whichever is greater.

It should be understood that these recommendations will help minimize the potential for soil movement and resulting distress, but will not eliminate this potential.

6.8 Corrosivity to Concrete

The chemical test results indicate that the soils at the site classify as Class S0 in accordance with Table 19.3.1.1 of ACI 318-19. However, in order to be consistent with standard local practice and for reasons of material availability, we recommend that Type II Portland cement be used for all concrete on and below grade.

6.9 Pavements

Based on existing subgrade conditions, the following pavement sections are recommended for the areas indicated:

Traffic Area	Asphalt Concrete (in.)	Base Course (in.)
Passenger car parking/drives (low traffic frequency)	3	7
Major access drives (high traffic frequency)	4	5

Bituminous surfacing should be constructed of dense-graded, central plant-mix, asphalt concrete. Base course and asphalt concrete should conform with City of Sedona current specifications.

Material and compaction requirements should conform to the recommendations presented under **EARTHWORK**. The gradient of paved surfaces should ensure positive drainage. Water should not pond in areas directly adjoining paved sections. The native subgrade soils will soften and lose stability if subjected to conditions which result in an increase in water content.

Due to the high static loads imposed by parked trucks in loading and unloading areas and at dumpster locations, we recommend that a rigid pavement section be considered for these areas. A minimum 6-inch thick concrete pavement over 4 inches of aggregate base course material is recommended.

The concrete should have a minimum 28-day compressive strength of 4,000 psi. Concrete quality will be important to produce the desired flexural strength and long-term durability. Assuming a nominal maximum aggregate size of 1 to 1-3/8 inches, the concrete is recommended to have entrained air content of 5 percent (+/- 1 percent) and a maximum water cement ratio of 0.45.

Proper joint placement and design is critical to pavement performance. Contraction joints should be placed at a maximum of 15 feet on-center. The contraction joints should be saw cut as soon as possible after placement of the concrete, but before shrinkage cracks occur. The concrete should be saw cut at least 3/8-inch wide and 2 inches deep.

Isolation joints should be placed where the pavement will abut the building, drainage inlets, manholes, T- and unsymmetrical intersections, and anywhere differential movement between the pavement and a structure may take place. The isolation joints should be 0.5 inch wide.

All joints should be properly cleaned and sealed as soon as possible to avoid infiltration of water, small gravel, and other debris. Either cold-pour or hot-poured sealing material may be used. Backing should be provided to hold the isolation joint sealant in place. Manufacturers' instructions for mixing and installing the joint materials should be followed.

6.9.1 Pavement Analyses

The recommended pavement sections are based on the following conditions. This firm should be contacted if any of these conditions change so that revised recommendations can be provided, if necessary.

a. A correlated R-value of 18 for the on-site soils which corresponds to a resilient modulus of approximately 6,500 pounds per square inch. Any required fills should be constructed using on-site or imported materials with subgrade support characteristics equal to or greater than the subgrade soils in the area being filled.

b. Structural coefficients of 0.40 for asphalt concrete and 0.12 for aggregate base course material.

c. A present serviceability index of 4.5, a terminal serviceability index of 2.5, an overall standard deviation of 0.35, a reliability factor of 85 percent, a drainage coefficient of 0.85, a seasonal variation factor of 2.4, and a design life of 20 years.

d. A total 18-kip equivalent single axle load (ESAL) of 25,000 for the passenger car parking areas and 50,000 for the major access drive areas.

7.0 EARTHWORK

7.1 General

The conclusions contained in this report for the proposed construction are contingent upon compliance with recommendations presented in this section. Any excavating, trenching, or disturbance that occurs after completion of the earthwork must be backfilled, compacted and tested in accordance with the recommendations contained herein. It is not reasonable to rely upon our conclusions and recommendations if any future unobserved and untested trenching, earthwork activities or backfilling occurs.

7.2 Site Clearing

Strip and remove all existing fill, vegetation, debris, and any other deleterious materials from the structure and pavement areas. The structure area is defined as that area within the footprint plus 5 feet beyond the perimeter of that footprint. All exposed surfaces should be free of mounds and depressions that could prevent uniform compaction.

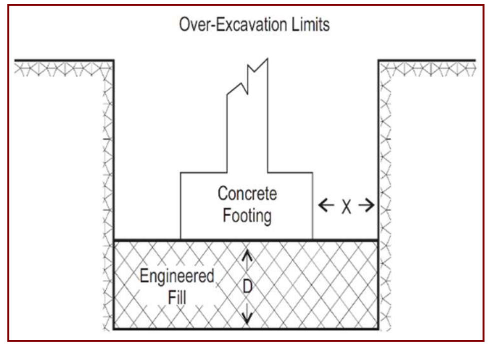
7.3 Excavation

We anticipate that excavations into the site soils for shallow foundations and utility trenches for the proposed construction can be accomplished with conventional equipment. Any deeper excavations penetrating the underlying dense sandstone may require the use of heavy-duty, specialized equipment to facilitate removal. The speed and ease of excavation is dependent on the nature of the deposit, the type of equipment used, and the skill and experience of the equipment operator. On-site soils may pump or become unworkable at high water contents. Workability may be improved by scarifying and drying. Over-excavation of wet zones and replacement with drier granular materials may be necessary. The use of lightweight excavation and compaction equipment may be required to minimize subgrade pumping.

7.4 Foundation Preparation

If all foundation elements are designed to bear on dense rock or on lean mix concrete backfill extending to dense rock, remove all loose or disturbed materials from the bottoms and sides of the excavations prior to placement of structural concrete or lean mix concrete backfill.

In footing areas, remove existing soils to a minimum depth of 1.5 feet below the bottom of the footing (depth D in the diagram). Removal should extend a minimum of 1.5 feet beyond the footing edges (length X in the diagram). Replace with engineered fill material.



7.5 Slab-on-Grade Preparation

Scarify, moisten or dry as required, and compact all subgrade soils to a minimum depth of 8 inches. The subgrade preparation should be accomplished in a manner which will result in uniform water contents and densities after compaction. All subgrade preparation in the building areas should extend a minimum of 5 feet beyond perimeter footings. In areas where dense sandstone is encountered, scarification and recompaction are not required.

7.6 Pavement Preparation

Prior to placement of fill and/or pavement materials, the exposed subgrade soils should be proof-rolled and observed by the geotechnical engineer or his qualified representative to verify that stable subgrade conditions exist. Any loose, soft, disturbed, or otherwise unsuitable materials should be over-excavated and replaced with engineered fill. The subgrade should then be scarified, moisture conditioned as required, and recompacted for a minimum depth of 8 inches. In areas where dense sandstone is encountered scarification and recompaction are not required.

7.7 Materials

a. Clean on-site soils with low expansive potentials and maximum dimension of 6 inches or imported materials may be used as fill material for the following:

- Foundation areas
- Slab-on-grade areas
- Pavement areas
- Backfill
- Landscape areas

b. Frozen soils should not be used as fill or backfill.

c. Lean mix (2-sack) concrete backfill should consist of aggregate base course type material combined with 2 sacks of cement per cubic yard. A coarse rock mix should not be used.

d. Imported soils should conform to the following:

- Gradation (ASTM C136): percent finer by weight

6"	100
4"	85-100
¾"	70-100
No. 4 Sieve	50-100
No. 200 Sieve	40 (max)

- Maximum expansive potential (%)[4] 1.5

- Maximum soluble sulfates (%) 0.10

e. Base course should conform to City of Sedona current specifications.

7.8 Placement and Compaction

a. Place and compact fill in horizontal lifts, using equipment and procedures that will produce recommended water contents and densities throughout the lift.

b. Uncompacted lift thickness should not exceed 8 inches.

[4] Measured on a sample compacted to approximately 95 percent of the ASTM D698 maximum dry density at about 3 percent below optimum water content. The sample is confined under a 100 psf surcharge and submerged.

c. Materials should be compacted to the following:

Material	Minimum Percent Compaction (ASTM D698)
Imported and on-site soil, fill and reworked:	
Below foundations	95
Below slabs-on-grade	95
Pavement areas	95
Aggregate base:	
Below slabs-on-grade	95
Pavement areas	100
Backfill:	
Structural	95
Nonstructural	90

d. On-site clay/clayey soils should be compacted with a moisture content in the range of 1 percent below to 3 percent above optimum. Imported soils with low expansive potential and aggregate base course materials should be compacted with a moisture content in the range of 3 percent below to 3 percent above optimum.

7.9 Compliance

Recommendations for foundations, slabs-on-grade and pavements supported on compacted fills or prepared subgrade depend upon compliance with the **EARTHWORK** recommendations. To assess compliance, observation and testing should be performed under the direction of a WT geotechnical engineer. Please contact us to provide these observation and testing services.

8.0 ADDITIONAL SERVICES

The recommendations provided in this report are based on the assumption that a sufficient schedule of tests and observations will be performed during construction to verify compliance. At a minimum, these tests and observations should be comprised of the following:

- Observations and testing during site preparation and earthwork,
- Observation of foundation excavations, and
- Consultation as may be required during construction.

Retaining the geotechnical engineer who developed your report to provide construction observation is the best way to verify compliance and to help you manage the risks associated with unanticipated conditions.

9.0 LIMITATIONS

This report has been prepared assuming the project criteria described in **2.0 PROJECT DESCRIPTION**. If changes in the project criteria occur, or if different subsurface conditions are encountered or become known, the conclusions and recommendations presented herein shall become invalid. In any such event, WT should be contacted in order to assess the effect that such variations may have on our conclusions and recommendations. If WT is not retained for the construction observation and testing services to determine compliance with this report, our professional responsibility is accordingly limited.

The recommendations presented are based entirely upon data derived from a limited number of samples obtained from widely spaced explorations. The attached logs are indicators of subsurface conditions only at the specific locations and times noted. This report assumes the uniformity of the geology and soil structure between explorations, however variations can and often do exist. Whenever any deviation, difference, or change is encountered or becomes known, WT should be contacted. This report is for the exclusive benefit of our client alone. There are no intended third-party beneficiaries of our contract with the client or this report, and nothing contained in the contract or this report shall create any express or implied contractual or any other relationship with, or claim or cause of action for, any third party against WT. This report is valid for the earlier of one year from the date of issuance, a change in circumstances, or discovered variations. After expiration, no person or entity shall rely on this report without the express written authorization of WT.

10.0 CLOSURE

We prepared this report as an aid to the designers of the proposed project. The comments, statements, recommendations and conclusions set forth in this report reflect the opinions of the authors. These opinions are based upon data obtained at the location of the explorations, and from laboratory tests. Work on your project was performed in accordance with generally accepted standards and practices utilized by professionals providing similar services in this locality. No other warranty, express or implied, is made.

Not to Scale

Approximate Test Boring Location

THE VILLAGE AT SADDLEROCK CROSSING	
Boring Location Diagram	
Western Technologies Inc.	
Job No.: 2523JB119	Plate: 1

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Allowable Soil Bearing Capacity	The recommended maximum contact stress developed at the interface of the foundation element and the supporting material.
Backfill	A specified material placed and compacted in a confined area.
Base Course	A layer of specified aggregate material placed on a subgrade or subbase.
Base Course Grade	Top of base course.
Bench	A horizontal surface in a sloped deposit.
Caisson/Drilled Shaft	A concrete foundation element cast in a circular excavation which may have an enlarged base (or belled caisson).
Concrete Slabs-On-Grade	A concrete surface layer cast directly upon base course, subbase or subgrade.
Crushed Rock Base Course	A base course composed of crushed rock of a specified gradation.
Differential Settlement	Unequal settlement between or within foundation elements of a structure.
Engineered Fill	Specified soil or aggregate material placed and compacted to specified density and/or moisture conditions under observations of a representative of a soil engineer.
Existing Fill	Materials deposited through the action of man prior to exploration of the site.
Existing Grade	The ground surface at the time of field exploration.
Expansive Potential	The potential of a soil to expand (increase in volume) due to absorption of moisture.
Fill	Materials deposited by the actions of man.
Finished Grade	The final grade created as a part of the project.
Gravel Base Course	A base course composed of naturally occurring gravel with a specified gradation.
Heave	Upward movement.
Native Grade	The naturally occurring ground surface.
Native Soil	Naturally occurring on-site soil.
Rock	A natural aggregate of mineral grains connected by strong and permanent cohesive forces. Usually requires drilling, wedging, blasting or other methods of extraordinary force for excavation.
Sand and Gravel Base Course	A base course of sand and gravel of a specified gradation.
Sand Base Course	A base course composed primarily of sand of a specified gradation.
Scarify	To mechanically loosen soil or break down existing soil structure.
Settlement	Downward movement.
Soil	Any unconsolidated material composed of discrete solid particles, derived from the physical and/or chemical disintegration of vegetable or mineral matter, which can be separated by gentle mechanical means such as agitation in water.
Strip	To remove from present location.
Subbase	A layer of specified material placed to form a layer between the subgrade and base course.
Subbase Grade	Top of subbase.
Subgrade	Prepared native soil surface.

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DEFINITION OF TERMINOLOGY

PLATE

A-1

COARSE-GRAINED SOILS
LESS THAN 50% FINES

GROUP SYMBOLS	DESCRIPTION	MAJOR DIVISIONS
GW	WELL-GRADED GRAVEL OR WELL-GRADED GRAVEL WITH SAND, LESS THAN 5% FINES	GRAVELS MORE THAN HALF OF COARSE FRACTION IS LARGER THAN NO. 4 SIEVE SIZE
GP	POORLY-GRADED GRAVEL OR POORLY-GRADED GRAVEL WITH SAND, LESS THAN 5% FINES	
GM	SILTY GRAVEL OR SILTY GRAVEL WITH SAND, MORE THAN 12% FINES	
GC	CLAYEY GRAVEL OR CLAYEY GRAVEL WITH SAND, MORE THAN 12% FINES	
SW	WELL-GRADED SAND OR WELL-GRADED SAND WITH GRAVEL, LESS THAN 5% FINES	SANDS MORE THAN HALF OF COARSE FRACTION IS SMALLER THAN NO. 4 SIEVE SIZE
SP	POORLY-GRADED SAND OR POORLY-GRADED SAND WITH GRAVEL, LESS THAN 5% FINES	
SM	SILTY SAND OR SILTY SAND WITH GRAVEL, MORE THAN 12% FINES	
SC	CLAYEY SAND OR CLAYEY SAND WITH GRAVEL, MORE THAN 12% FINES	

NOTE: Coarse-grained soils receive dual symbols if they contain 5% to 12% fines (e.g., SW-SM, GP-GC).

FINE-GRAINED SOILS
MORE THAN 50% FINES

GROUP SYMBOLS	DESCRIPTION	MAJOR DIVISIONS
ML	SILT, SILT WITH SAND OR GRAVEL, SANDY SILT, OR GRAVELLY SILT	SILTS AND CLAYS LIQUID LIMIT LESS THAN 50
CL	LEAN CLAY OF LOW TO MEDIUM PLASTICITY, SANDY CLAY, OR GRAVELLY CLAY	
OL	ORGANIC SILT OR ORGANIC CLAY OF LOW TO MEDIUM PLASTICITY	
MH	ELASTIC SILT, SANDY ELASTIC SILT, OR GRAVELLY ELASTIC SILT	SILTS AND CLAYS LIQUID LIMIT MORE THAN 50
CH	FAT CLAY OF HIGH PLASTICITY, SANDY FAT CLAY, OR GRAVELLY FAT CLAY	
OH	ORGANIC SILT OR ORGANIC CLAY OF HIGH PLASTICITY	
PT	PEAT AND OTHER HIGHLY ORGANIC SOILS	HIGHLY ORGANIC SOILS

NOTE: Fine-grained soils may receive dual classification based upon plasticity characteristics (e.g. CL-ML).

SOIL SIZES

COMPONENT	SIZE RANGE
BOULDERS	Above 12 in.
COBBLES	3 in. – 12 in.
GRAVEL	No. 4 – 3 in.
Coarse	¾ in. – 3 in.
Fine	No. 4 – ¾ in.
SAND	No. 200 – No. 4
Coarse	No. 10 – No. 4
Medium	No. 40 – No. 10
Fine	No. 200 – No. 40
Fines (Silt or Clay)	Below No. 200

NOTE: Only sizes smaller than three inches are used to classify soils

CONSISTENCY

CLAYS & SILTS	BLOWS PER FOOT
VERY SOFT	0 – 2
SOFT	3 – 4
FIRM	5 – 8
STIFF	9 – 15
VERY STIFF	16 – 30
HARD	OVER 30

RELATIVE DENSITY

SANDS & GRAVELS	BLOWS PER FOOT
VERY LOOSE	0 – 4
LOOSE	5 – 10
MEDIUM DENSE	11 – 30
DENSE	31 – 50
VERY DENSE	OVER 50

NOTE: Number of blows using 140-pound hammer falling 30 inches to drive a 2-inch-OD (1⅜-inch ID) split-barrel sampler (ASTM D1586).

PLASTICITY OF FINE GRAINED SOILS

PLASTICITY INDEX	TERM
0	NON-PLASTIC
1 – 7	LOW
8 – 20	MEDIUM
Over 20	HIGH

DEFINITION OF WATER CONTENT

DRY
SLIGHTLY DAMP
DAMP
MOIST
WET
SATURATED

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METHOD OF CLASSIFICATION

PLATE
A-2

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The number shown in **"BORING NO."** refers to the approximate location of the same number indicated on the "Boring Location Diagram" as positioned in the field by pacing or measurement from property lines and/or existing features.

"DRILLING TYPE" refers to the exploratory equipment used in the boring wherein **HSA = hollow stem auger**, and the dimension presented is the outside diameter of the HSA used.

"R" in "BLOW COUNTS" refers to a 3-inch outside diameter ring-lined split barrel sampler driven into the ground with a 140 pound drop-hammer dropped 30 inches repeatedly until a penetration of 12 inches is achieved or until refusal. The number of blows required to advance the sampler 12 inches is defined as the "**R**" blow count. The "**R**" blow count requires an engineered conversion to an equivalent SPT N-Value. Refusal to penetration is considered more than 50 blows per foot. An **X** within the symbol indicates no sample recovery. A circle within the symbol indicates sample disturbance.

"SAMPLE TYPE" refers to the form of sample recovery, in which **R** = Ring-lined sample and **G** = Grab sample.

"DRY DENSITY (LBS/CU FT)" refers to the laboratory-determined dry density in pounds per cubic foot.

"WATER (MOISTURE) CONTENT" (% of Dry Wt.) refers to the laboratory-determined water content in percent using the standard test method ASTM D2216.

"USCS" refers to the "Unified Soil Classification System" Group Symbol for the soil type as defined by ASTM D2487 and D2488. The soils were classified visually in the field, and where appropriate, classifications were modified by visual examination of samples in the laboratory and/or by appropriate tests.

These notes and boring logs are intended for use in conjunction with the purposes of our services defined in the text. Boring log data should not be construed as part of the construction plans nor as defining construction conditions.

Boring logs depict our interpretations of subsurface conditions at the locations and on the date(s) noted. Variations in subsurface conditions and characteristics may occur between borings. Groundwater levels may fluctuate due to seasonal variations and other factors.

The stratification lines shown on the boring logs represent our interpretation of the approximate boundary between soil or rock types based upon visual field classification at the boring location. The transition between materials is approximate and may be more or less gradual than indicated.

Western Technologies An RMA Company	BORING LOG NOTES	PLATE A-3

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Project: The Village at Saddlerock Crossing Project Number: 2523JB119	BORING NO. 1	Western Technologies An RMA Company
Date(s) Drilled **6/29/23**	Logged By **C. Bartlett**	Checked By **J. Quinlan**
Drilling Method **HSA**	Drill Bit Size/Type **7 IN.**	Approximate Surface Elevation **NOT DETERMINED**
Drill Rig Type **CME-75**	Drilling Contractor **EDI**	
Groundwater Level and Date Measured **NOT ENCOUNTERED**	Location **SEE LOCATION DIAGRAM**	

Water Content, %	Dry Unit Weight, pcf	Sample	Sampling Resistance, blows/ft	Depth (feet)	Soil Type	MATERIAL DESCRIPTION
		G		0	CL-ML	Sandy, Silty CLAY; some gravel, red, stiff to hard, damp
11.0	107	R	15			
18.9	99	R	50/9"	5		
				9		Auger Refusal at 9 Feet on SANDSTONE
				10		
				15		
				20		
				25		

Figure A-4

Project: The Village at Saddlerock Crossing Project Number: 2523JB119	BORING NO. 2	Western Technologies An RMA Company

Date(s) Drilled **6/29/23**	Logged By **C. Bartlett**	Checked By **J. Quinlan**
Drilling Method **HSA**	Drill Bit Size/Type **7 IN.**	Approximate Surface Elevation **NOT DETERMINED**
Drill Rig Type **CME-75**	Drilling Contractor **EDI**	
Groundwater Level and Date Measured **NOT ENCOUNTERED**	Location **SEE LOCATION DIAGRAM**	

Water Content, %	Dry Unit Weight, pcf	Sample Type	Sample	Sampling Resistance, blows/ft	Depth (feet)	Soil Type	Graphic Log	MATERIAL DESCRIPTION
			G		0	CL		Lean CLAY; with sand, trace gravel, red, stiff to hard, damp
11.2	106		R	15				
12.2	108		R	34	5			
					10			
14.6	102		R	50/2"				
					11			Auger Refusal at 11 Feet on SANDSTONE
					15			
					20			
					25			

Figure A-5

Project: The Village at Saddlerock Crossing Project Number: 2523JB119	BORING NO. 3	Western Technologies An RMA Company
Date(s) Drilled **6/29/23**	Logged By **C. Bartlett**	Checked By **J. Quinlan**
Drilling Method **HSA**	Drill Bit Size/Type **7 IN.**	Approximate Surface Elevation **NOT DETERMINED**
Drill Rig Type **CME-75**	Drilling Contractor **EDI**	
Groundwater Level and Date Measured **NOT ENCOUNTERED**	Location **SEE LOCATION DIAGRAM**	

Water Content, %	Dry Unit Weight, pcf	Sample Type	Sample	Sampling Resistance, blows/ft	Depth (feet)	Soil Type	Graphic Log	MATERIAL DESCRIPTION
			G		0	CL-ML		Sandy, Silty CLAY; some gravel, red, hard, slightly damp
8.1	110		R	32				
					4			Auger Refusal at 4 Feet on SANDSTONE
					5			
					10			
					15			
					20			
					25			

Figure A-6

Project: The Village at Saddlerock Crossing Project Number: 2523JB119	BORING NO. 4	Western Technologies An RMA Company

Date(s) Drilled 6/29/23	Logged By C. Bartlett	Checked By J. Quinlan
Drilling Method HSA	Drill Bit Size/Type 7 IN.	Approximate Surface Elevation NOT DETERMINED
Drill Rig Type CME-75	Drilling Contractor EDI	
Groundwater Level and Date Measured NOT ENCOUNTERED	Location SEE LOCATION DIAGRAM	

Water Content, %	Dry Unit Weight, pcf	Sample Type	Sample	Sampling Resistance, blows/ft	Depth (feet)	Soil Type	Graphic Log	MATERIAL DESCRIPTION
			G		0	CL		Lean CLAY; with sand, trace gravel, red, stiff to hard, damp to slightly damp
10.0	107		R	9				
5.7	98		R	16	5			
5.7	110		R	50/2"	10			
9.7	95		R	50/1"	15			
					16			Auger Refusal at 16 Feet on SANDSTONE
					20			
					25			

Figure A-7

Project: The Village at Saddlerock Crossing

Project Number: 2523JB119

BORING NO. 5

Western Technologies
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Date(s) Drilled **6/29/23**	Logged By **C. Bartlett**	Checked By **J. Quinlan**
Drilling Method **HSA**	Drill Bit Size/Type **7 IN.**	Approximate Surface Elevation **NOT DETERMINED**
Drill Rig Type **CME-75**	Drilling Contractor **EDI**	
Groundwater Level and Date Measured **NOT ENCOUNTERED**	Location **SEE LOCATION DIAGRAM**	

Water Content, %	Dry Unit Weight, pcf	Sample	Sampling Resistance, blows/ft	Depth (feet)	Soil Type	MATERIAL DESCRIPTION
		G		0	CL	Lean CLAY; with sand, trace gravel, red, stiff to hard, damp
13.1	112	R	14			
10.8	91	R	50/2"	5		
				6		Auger Refusal at 6 Feet on SANDSTONE

Depth scale: 0, 5, 10, 15, 20, 25

Figure A-8

Project: The Village at Saddlerock Crossing Project Number: 2523JB119	BORING NO. 6	Western Technologies An RMA Company

Date(s) Drilled **6/29/23**	Logged By **C. Bartlett**	Checked By **J. Quinlan**
Drilling Method **HSA**	Drill Bit Size/Type **7 IN.**	Approximate Surface Elevation **NOT DETERMINED**
Drill Rig Type **CME-75**	Drilling Contractor **EDI**	
Groundwater Level and Date Measured **NOT ENCOUNTERED**	Location **SEE LOCATION DIAGRAM**	

Water Content, %	Dry Unit Weight, pcf	Sample Type	Sample	Sampling Resistance, blows/ft	Depth (feet)	Soil Type	Graphic Log	MATERIAL DESCRIPTION
			G		0	CL		Lean CLAY; with sand, trace gravel, red/brown, very stiff, damp
11.9	113		R	23				
					5			
					9			Auger Refusal at 9 Feet on SANDSTONE
					10			
					15			
					20			
					25			

Figure A-9

Project: The Village at Saddlerock Crossing Project Number: 2523JB119	BORING NO. 7	Western Technologies An RMA Company

Date(s) Drilled **6/29/23**	Logged By **C. Bartlett**	Checked By **J. Quinlan**
Drilling Method **HSA**	Drill Bit Size/Type **7 IN.**	Approximate Surface Elevation **NOT DETERMINED**
Drill Rig Type **CME-75**	Drilling Contractor **EDI**	
Groundwater Level and Date Measured **NOT ENCOUNTERED**	Location **SEE LOCATION DIAGRAM**	

Water Content, %	Dry Unit Weight, pcf	Sample Type	Sample	Sampling Resistance, blows/ft	Depth (feet)	Soil Type	Graphic Log	MATERIAL DESCRIPTION
			G		0	CL-ML		Sandy, Silty CLAY; some gravel, red, stiff to hard, damp
14.1	114		R	15				
11.7	95		R	50/1"	5			
					6			Auger Refusal at 6 Feet on SANDSTONE
					10			
					15			
					20			
					25			

Project: The Village at Saddlerock Crossing Project Number: 2523JB119	BORING NO. 8	Western Technologies An RMA Company
Date(s) Drilled **6/29/23**	Logged By **C. Bartlett**	Checked By **J. Quinlan**
Drilling Method **HSA**	Drill Bit Size/Type **7 IN.**	Approximate Surface Elevation **NOT DETERMINED**
Drill Rig Type **CME-75**	Drilling Contractor **EDI**	
Groundwater Level and Date Measured **NOT ENCOUNTERED**	Location **SEE LOCATION DIAGRAM**	

Water Content, %	Dry Unit Weight, pcf	Sample Type	Sample	Sampling Resistance, blows/ft	Depth (feet)	Soil Type	Graphic Log	MATERIAL DESCRIPTION
			G		0	CL-ML		Sandy, Silty CLAY; some gravel, red, very stiff to hard, damp to slightly damp
11.0	101		R	18				
7.9	105		R	50/11"	5			
					9			Auger Refusal at 9 Feet on SANDSTONE
					10			
					15			
					20			
					25			

Figure A-11

Project: The Village at Saddlerock Crossing Project Number: 2523JB119	BORING NO. 9	Western Technologies An RMA Company
Date(s) Drilled **6/29/23**	Logged By **C. Bartlett**	Checked By **J. Quinlan**
Drilling Method **HSA**	Drill Bit Size/Type **7 IN.**	Approximate Surface Elevation **NOT DETERMINED**
Drill Rig Type **CME-75**	Drilling Contractor **EDI**	
Groundwater Level and Date Measured **NOT ENCOUNTERED**	Location **SEE LOCATION DIAGRAM**	

Water Content, %	Dry Unit Weight, pcf	Sample	Sampling Resistance, blows/ft	Depth (feet)	Soil Type	MATERIAL DESCRIPTION
		G		0	CL-ML	Sandy, Silty CLAY; some gravel, red, very stiff to hard, slightly damp
8.0	121	R	21			
6.8	107	R	50/10"	5		
6.7	117	R	50/8"	10		
3.6	100	R	50/2"	15		
		R	50/2"	18		Boring Stopped at 18 Feet
				20		
				25		

Project: The Village at Saddlerock Crossing Project Number: 2523JB119	BORING NO. 10	Western Technologies An RMA Company

Date(s) Drilled **6/29/23**	Logged By **C. Bartlett**	Checked By **J. Quinlan**
Drilling Method **HSA**	Drill Bit Size/Type **7 IN.**	Approximate Surface Elevation **NOT DETERMINED**
Drill Rig Type **CME-75**	Drilling Contractor **EDI**	
Groundwater Level and Date Measured **NOT ENCOUNTERED**	Location **SEE LOCATION DIAGRAM**	

Water Content, %	Dry Unit Weight, pcf	Sample Type	Sample	Sampling Resistance, blows/ft	Depth (feet)	Soil Type	Graphic Log	MATERIAL DESCRIPTION
			G		0	CL-ML		Sandy, Silty CLAY; some gravel, red, very stiff, slightly damp
7.2	112		R	26				
6.0	100		R	28	5			
					7			Auger Refusal at 7 Feet on SANDSTONE
					10			
					15			
					20			
					25			

Figure A-13

Project: The Village at Saddlerock Crossing Project Number: 2523JB119	BORING NO. 11	Western Technologies An RMA Company
Date(s) Drilled **6/29/23**	Logged By **C. Bartlett**	Checked By **J. Quinlan**
Drilling Method **HSA**	Drill Bit Size/Type **7 IN.**	Approximate Surface Elevation **NOT DETERMINED**
Drill Rig Type **CME-75**	Drilling Contractor **EDI**	
Groundwater Level and Date Measured **NOT ENCOUNTERED**	Location **SEE LOCATION DIAGRAM**	

Water Content, %	Dry Unit Weight, pcf	Sample Type	Sample	Sampling Resistance, blows/ft	Depth (feet)	Soil Type	Graphic Log	MATERIAL DESCRIPTION
			G		0	CL-ML		Sandy, Silty CLAY; some gravel, red, firm to hard, slightly damp
6.0	93		R	7				
6.8	82		R	50/3"	5			
								Auger Refusal at 6 Feet on SANDSTONE
					10			
					15			
					20			
					25			

Figure A-14

Project: The Village at Saddlerock Crossing Project Number: 2523JB119	BORING NO. 12	Western Technologies An RMA Company

Date(s) Drilled **6/29/23**	Logged By **C. Bartlett**	Checked By **J. Quinlan**
Drilling Method **HSA**	Drill Bit Size/Type **7 IN.**	Approximate Surface Elevation **NOT DETERMINED**
Drill Rig Type **CME-75**	Drilling Contractor **EDI**	
Groundwater Level and Date Measured **NOT ENCOUNTERED**	Location **SEE LOCATION DIAGRAM**	

Water Content, %	Dry Unit Weight, pcf	Sample Type	Sample	Sampling Resistance, blows/ft	Depth (feet)	Soil Type	Graphic Log	MATERIAL DESCRIPTION
			G		0	CL-ML		Sandy, Silty CLAY; some gravel, red, hard, slightly damp
5.8	113		R	50/11"				
7.8	96		R	50/10"	5			
6.0	90		R	50/2"	10			
5.6	107		R	50/1"	15			
					17			Auger Refusal at 17 Feet on SANDSTONE
					20			
					25			

Project: The Village at Saddlerock Crossing **Project Number: 2523JB119**	**BORING NO. 13**	Western Technologies An RMA Company

Date(s) Drilled **6/29/23**	Logged By **C. Bartlett**	Checked By **J. Quinlan**
Drilling Method **HSA**	Drill Bit Size/Type **7 IN.**	Approximate Surface Elevation **NOT DETERMINED**
Drill Rig Type **CME-75**	Drilling Contractor **EDI**	
Groundwater Level and Date Measured **NOT ENCOUNTERED**	Location **SEE LOCATION DIAGRAM**	

Water Content, %	Dry Unit Weight, pcf	Sample	Sampling Resistance, blows/ft	Depth (feet)	Soil Type	MATERIAL DESCRIPTION
		G		0	GM-GC	Silty, Clayey GRAVEL; with sand, red, dense, slightly damp
4.2	120	R	41			
15.4	102	R	50/10"	5		
					CL-ML	Sandy, Silty CLAY; some gravel, red, hard, damp
15.6	111	R	50/1"	10		
11.1	102	R	50/10"	15		
5.9	115	R	50/10"	20		
				21		Boring Stopped at 21 Feet
				25		

Figure A-16

Project: The Village at Saddlerock Crossing **Project Number: 2523JB119**	**BORING NO. 14**	Western Technologies An RMA Company

Date(s) Drilled **6/29/23**	Logged By **C. Bartlett**	Checked By **J. Quinlan**
Drilling Method **HSA**	Drill Bit Size/Type **7 IN.**	Approximate Surface Elevation **NOT DETERMINED**
Drill Rig Type **CME-75**	Drilling Contractor **EDI**	
Groundwater Level and Date Measured **NOT ENCOUNTERED**	Location **SEE LOCATION DIAGRAM**	

Water Content, %	Dry Unit Weight, pcf	Sample Type	Sample	Sampling Resistance, blows/ft	Depth (feet)	Soil Type	Graphic Log	MATERIAL DESCRIPTION
			G		0	ML		Sandy SILT; some gravel, red/brown, soft, damp
11.0	95		R	4				
					5			Boring Stopped at 5 Feet
					10			
					15			
					20			
					25			

Figure A-17

Project: The Village at Saddlerock Crossing Project Number: 2523JB119	BORING NO. 15	Western Technologies An RMA Company
Date(s) Drilled **6/29/23**	Logged By **C. Bartlett**	Checked By **J. Quinlan**
Drilling Method **HSA**	Drill Bit Size/Type **7 IN.**	Approximate Surface Elevation **NOT DETERMINED**
Drill Rig Type **CME-75**	Drilling Contractor **EDI**	
Groundwater Level and Date Measured **NOT ENCOUNTERED**	Location **SEE LOCATION DIAGRAM**	

Water Content, %	Dry Unit Weight, pcf	Sample Type	Sample	Sampling Resistance, blows/ft	Depth (feet)	Soil Type	Graphic Log	MATERIAL DESCRIPTION
			G		0	CL-ML		Sandy, Silty CLAY; some gravel, red/brown, stiff, damp
15.2	109		R	10				
					5			Boring Stopped at 5 Feet
					10			
					15			
					20			
					25			

Figure A-18

Project: The Village at Saddlerock Crossing Project Number: 2523JB119	**BORING NO. 16**	Western Technologies An RMA Company

Date(s) Drilled **6/29/23**	Logged By **C. Bartlett**	Checked By **J. Quinlan**
Drilling Method **HSA**	Drill Bit Size/Type **7 IN.**	Approximate Surface Elevation **NOT DETERMINED**
Drill Rig Type **CME-75**	Drilling Contractor **EDI**	
Groundwater Level and Date Measured **NOT ENCOUNTERED**	Location **SEE LOCATION DIAGRAM**	

Water Content, %	Dry Unit Weight, pcf	Sample Type	Sample	Sampling Resistance, blows/ft	Depth (feet)	Soil Type	Graphic Log	MATERIAL DESCRIPTION
			G		0	CL-ML		Sandy, Silty CLAY; some gravel, red, hard, slightly damp
5.2	110		R	31				
9.5	114		R	39	5			
					8			Auger Refusal at 8 Feet on SANDSTONE
					10			
					15			
					20			
					25			

Figure A-19

Project: The Village at Saddlerock Crossing Project Number: 2523JB119	BORING NO. 17	Western Technologies An RMA Company
Date(s) Drilled 6/29/23	Logged By C. Bartlett	Checked By J. Quinlan
Drilling Method HSA	Drill Bit Size/Type 7 IN.	Approximate Surface Elevation NOT DETERMINED
Drill Rig Type CME-75	Drilling Contractor EDI	
Groundwater Level and Date Measured NOT ENCOUNTERED	Location SEE LOCATION DIAGRAM	

Water Content, %	Dry Unit Weight, pcf	Sample Type	Sample	Sampling Resistance, blows/ft	Depth (feet)	Soil Type	Graphic Log	MATERIAL DESCRIPTION
			G		0	CL-ML		Sandy, Silty CLAY; some gravel, red, stiff to very stiff, damp
13.7	108		R	11				
14.5	112		R	17	5			
					7			Auger Refusal at 7 Feet on SANDSTONE
					10			
					15			
					20			
					25			

Figure A-20

Project: The Village at Saddlerock Crossing **Project Number: 2523JB119**	**BORING NO. 18**	Western Technologies An RMA Company

Date(s) Drilled **6/29/23**	Logged By **C. Bartlett**	Checked By **J. Quinlan**
Drilling Method **HSA**	Drill Bit Size/Type **7 IN.**	Approximate Surface Elevation **NOT DETERMINED**
Drill Rig Type **CME-75**	Drilling Contractor **EDI**	
Groundwater Level and Date Measured **NOT ENCOUNTERED**	Location **SEE LOCATION DIAGRAM**	

Water Content, %	Dry Unit Weight, pcf	Sample Type	Sample	Sampling Resistance, blows/ft	Depth (feet)	Soil Type	Graphic Log	MATERIAL DESCRIPTION
			G		0	CL-ML		Sandy, Silty CLAY; some gravel, red, hard, slightly damp
2.2	108		R	31				
					5			Boring Stopped at 5 Feet
					10			
					15			
					20			
					25			

Figure A-21

Project: The Village at Saddlerock Crossing Project Number: 2523JB119	BORING NO. 19	Western Technologies An RMA Company

Date(s) Drilled **6/29/23**	Logged By **C. Bartlett**	Checked By **J. Quinlan**
Drilling Method **HSA**	Drill Bit Size/Type **7 IN.**	Approximate Surface Elevation **NOT DETERMINED**
Drill Rig Type **CME-75**	Drilling Contractor **EDI**	
Groundwater Level and Date Measured **NOT ENCOUNTERED**	Location **SEE LOCATION DIAGRAM**	

Water Content, %	Dry Unit Weight, pcf	Sample Type	Sample	Sampling Resistance, blows/ft	Depth (feet)	Soil Type	Graphic Log	MATERIAL DESCRIPTION
			G		0	CL-ML		Sandy, Silty CLAY; some gravel, red, firm, slightly damp
2.8	96		R	5				
					5			Boring Stopped at 5 Feet
					10			
					15			
					20			
					25			

Figure A-22

Boring No.	Depth (ft)	USCS Class.	Particle Size Distribution (%) Passing by Weight							Atterberg Limits		Laboratory Compaction Characteristics			Remarks
			3"	¾"	#4	#10	#40	#200	2µ	LL	PI	Dry Density (pcf)	Optimum Moisture (%)	Method	
1	0-5	CL-ML	100	99	88	81	74	56.0		23	6				2
5	0-5	CL		100	97	93	86	71.5		25	10				2
8	0-5	CL-ML	100	99	89	84	79	61.5		23	7				2
11	0-5	CL-ML	100	99	95	93	91	62.3		19	4				2
13	0-5	GC-GM	100	86	71	64	59	47.6		21	4				2
14	0-5	ML	100	97	91	88	83	66.5		19	3				2
16	0-5	CL-ML		100	99	97	96	69.3		22	5				2

NOTE: NP = Non-plastic
µ = microns (2µ = 0.002mm)

REMARKS

Classification / Particle Size / Moisture-Density Relationship
1. Visual
2. Laboratory Tested
3. Minus #200 Only
4. Test Method ASTM D698/AASHTO T99
5. Test Method ASTM D1557/AASHTO T180
6. From the ADOT Family of Curves

Western Technologies
An RMA Company

PROJECT: THE VILLAGE AT SADDLEROCK CROSSING
JOB NO.: 2523JB119

PLATE
B-1

SOIL PROPERTIES

Boring No.	Depth (ft.)	USCS Class.	Initial Dry Density (pcf)	Initial Water Content (%)	Laboratory Compaction Characteristics			Expansion Properties		Plasticity		Soluble		Remarks
					Dry Density(pcf)	Optimum Moisture(%)	Method	Surcharge (ksf)	Expansion (%)	LL	PI	Salts (ppm)	Sulfate (ppm)	
1	0-5	CL-ML	112.5	9.2	119.4	11.7	A	0.1	0.2					1,2,3
11	0-5	CL-ML	110.3	8.3				0.1	0					1,2
13	0-5	GC-GM	112.8	8.1	119.1	10.7	A	0.1	0					1,2,3

Notes: Initial Dry Density and Initial Water Content are remolded.

Remarks

1. Compacted density (approx. 95% of ASTM D698 max. density at moisture content slightly below optimum.)
2. Submerged to approximate saturation.
3. Test Method ASTM D698/AASHTO T99
4. Test Method ASTM D1557/AASHTO T180
5. From the ADOT Family of Curves

Western Technologies
An RMA Company

PROJECT: THE VILLAGE AT SADDLEROACK CROSSING
JOB NO.: 2523JB119

SOIL PROPERTIES

PLATE
B-2

MOTZZ LABORATORY, INC.

Report: 947305
Reported: 7/12/2023
Received: 7/10/2023
PO: 2523P038

Laboratory Analysis Report

Western Technologies - Flagstaff
Gregory L. E. Burr
2400 East Huntington
Flagstaff, AZ 86004-8934

Project: 2523JB119

Lab Number	Sample ID
947305-1	8 (0-5')

Test Parameter

Test	*Method*	*Result*	*Units*
Soluble Salts	ARIZ 237b	794	ppm
Sulfate	ARIZ 733b	5	ppm
Chloride	ARIZ 736b	57	ppm

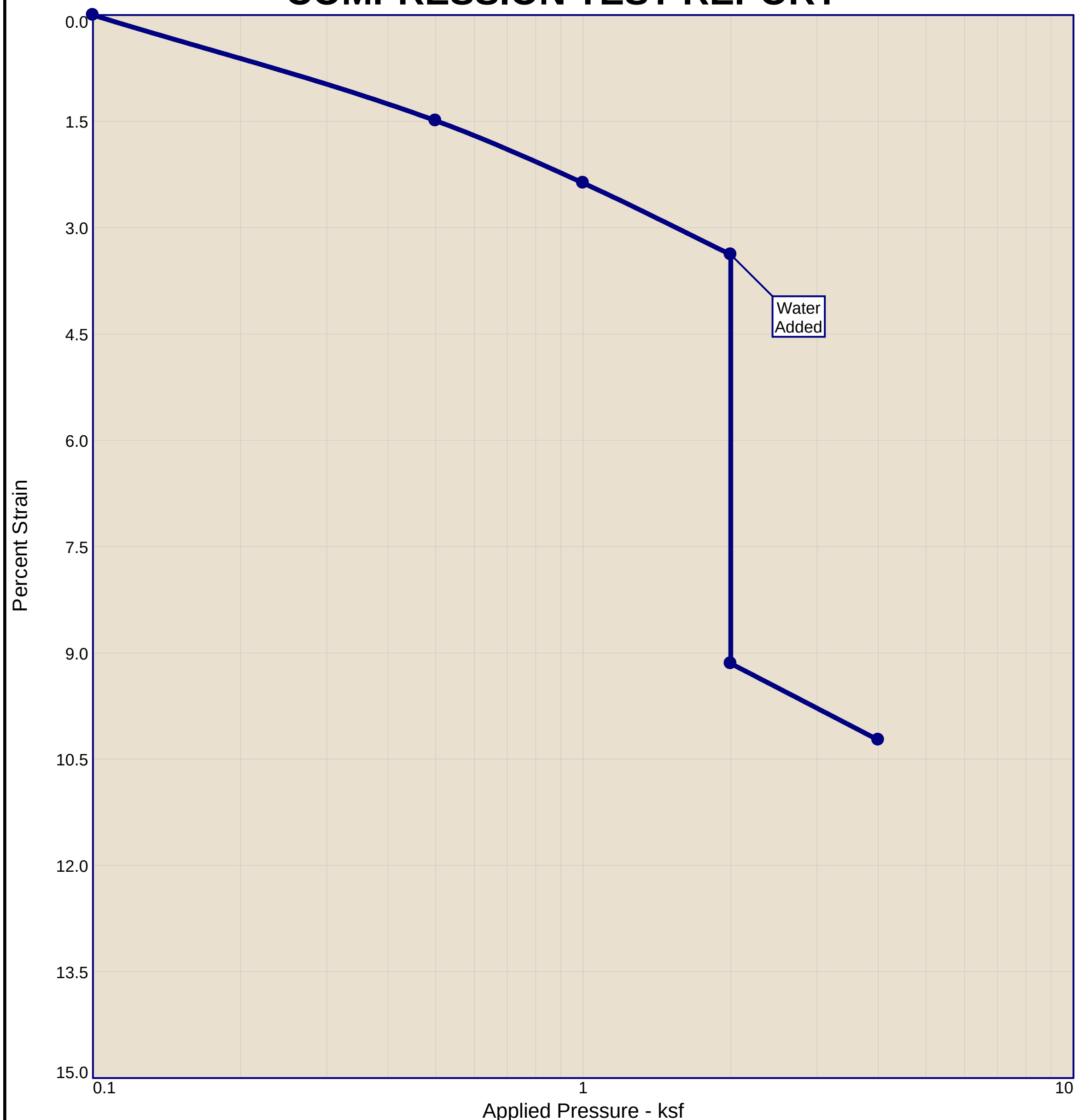
Lab Number	Sample ID
947301-1	13 (0-5')

Test Parameter

Test	*Method*	*Result*	*Units*
Soluble Salts	ARIZ 237b	352	ppm
Sulfate	ARIZ 733b	4	ppm
Chloride	ARIZ 736b	17	ppm

Plate B-3

COMPRESSION TEST REPORT



Natural Sat.	Natural Moist.	Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (ksf)	P_c (ksf)	C_c	C_r	Swell Press. (ksf)	Clpse. %	e_o
29.9 %	5.7 %	109.7			2.65						5.8	0.509

MATERIAL DESCRIPTION	USCS	AASHTO
LEAN CLAY WITH SAND	CL	

Project No. 2523JB119 **Client:** MR. CURT BANEY

Project: THE VILLAGE AT SADDLEROCK CROSSING

Source: RING SAMPLE **Depth:** 10-11 FEET **Sample No.:** BORING 4

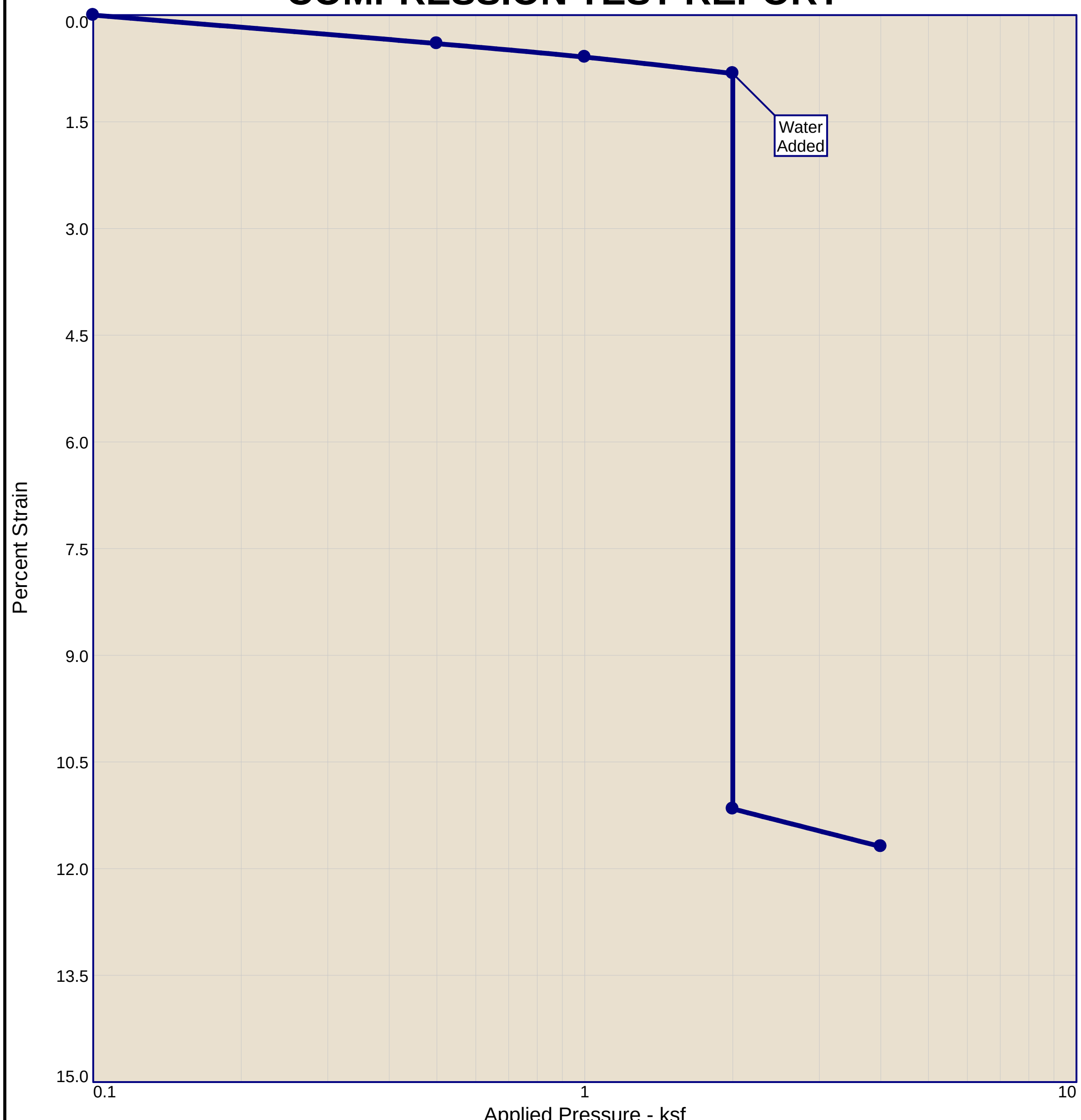
Remarks:

Western Technologies, Inc.

Flagstaff, AZ

Plate B-4

COMPRESSION TEST REPORT



Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (ksf)	P_c (ksf)	C_c	C_r	Swell Press. (ksf)	Clpse. %	e_o
Sat.	Moist.											
34.3 %	9.7 %	94.5			2.65						10.3	0.751

MATERIAL DESCRIPTION	USCS	AASHTO
LEAN CLAY WITH SAND	CL	

Project No. 2523JB119 **Client:** MR. CURT BANEY

Project: THE VILLAGE AT SADDLEROCK CROSSING

Source: RING SAMPLE **Depth:** 15-16 FEET **Sample No.:** BORING 4

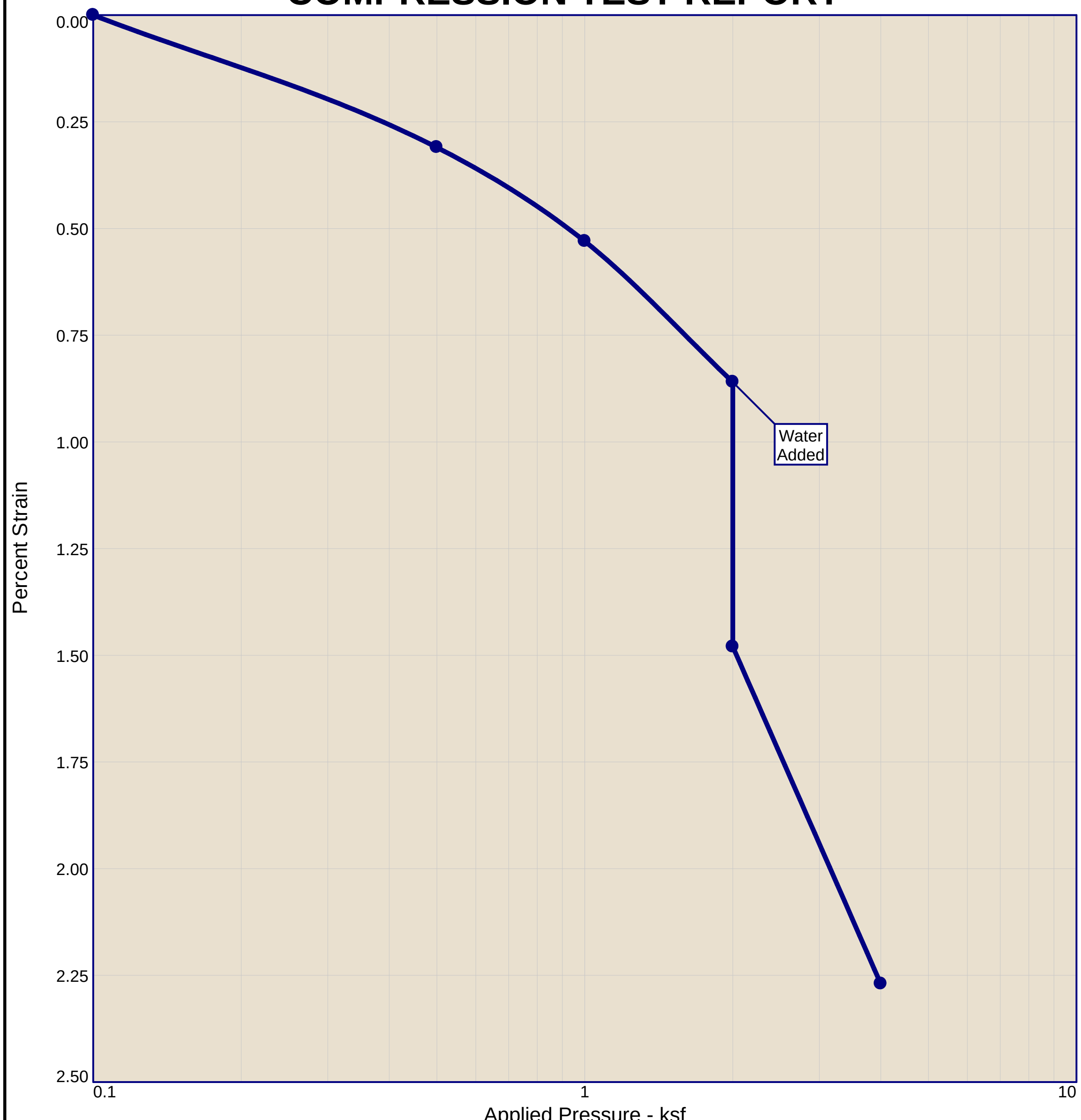
Western Technologies, Inc.

Flagstaff, AZ

Remarks:

Plate B-5

COMPRESSION TEST REPORT



Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (ksf)	P_c (ksf)	C_c	C_r	Swell Press. (ksf)	Clpse. %	e_o
Sat.	Moist.											
71.8 %	13.1 %	111.6			2.65						0.6	0.482

MATERIAL DESCRIPTION	USCS	AASHTO
LEAN CLAY WITH SAND	CL	

Project No. 2523JB119 **Client:** MR. CURT BANEY

Project: THE VILLAGE AT SADDLEROCK CROSSING

Source: RING SAMPLE **Depth:** 2-3 FEET **Sample No.:** BORING 5

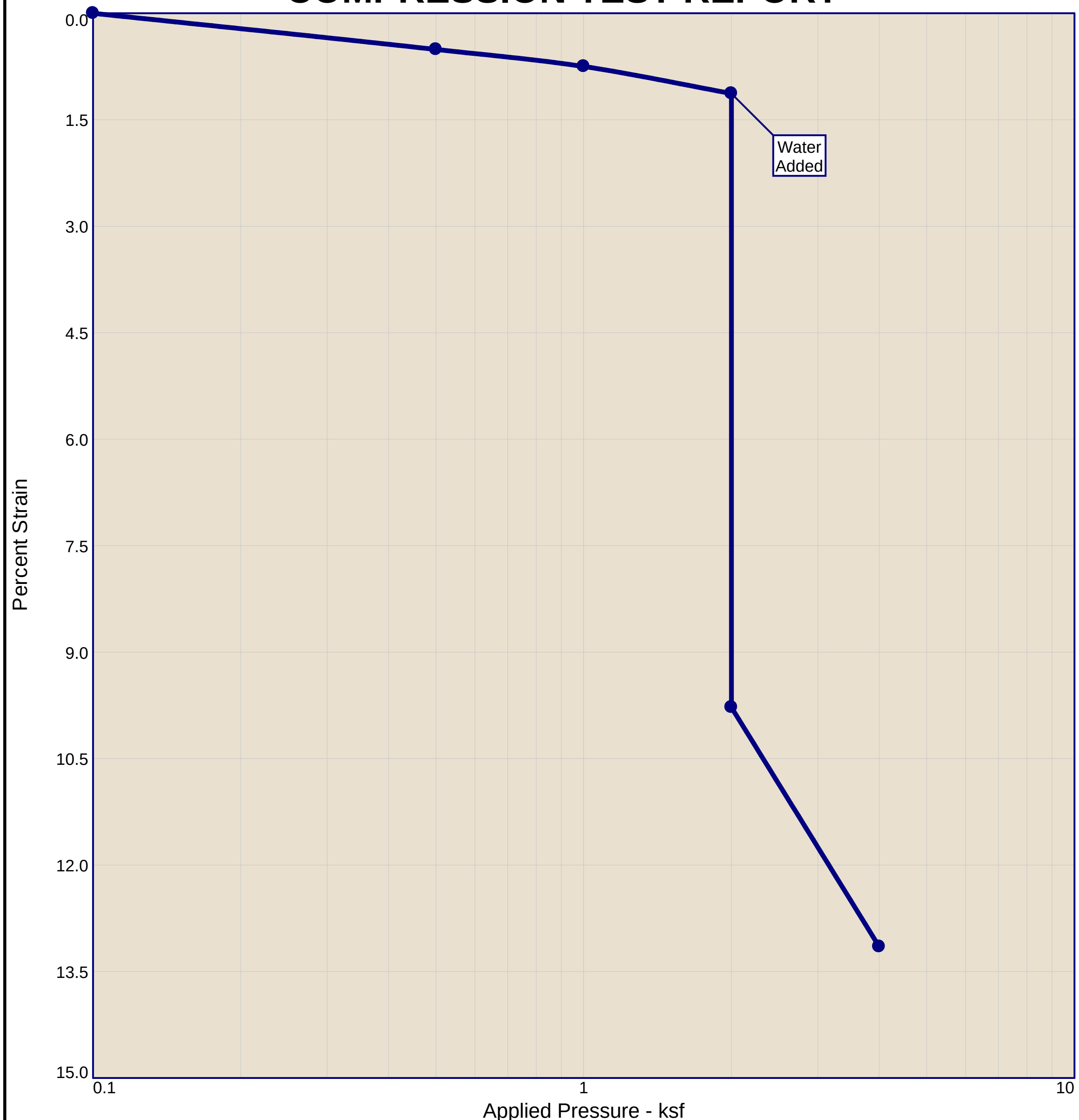
Western Technologies, Inc.

Flagstaff, AZ

Remarks:

Plate B-6

COMPRESSION TEST REPORT



Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (ksf)	P_c (ksf)	C_c	C_r	Swell Press. (ksf)	Clpse. %	e_o
Sat.	Moist.											
41.7 %	11.7 %	94.8			2.65						8.7	0.744

MATERIAL DESCRIPTION	USCS	AASHTO
SANDY, SILTY CLAY	CL-ML	

Project No. 2523JB119 **Client:** MR. CURT BANEY

Project: THE VILLAGE AT SADDLEROCK CROSSING

Source: RING SAMPLE **Depth:** 5-6 FEET **Sample No.:** BORING 7

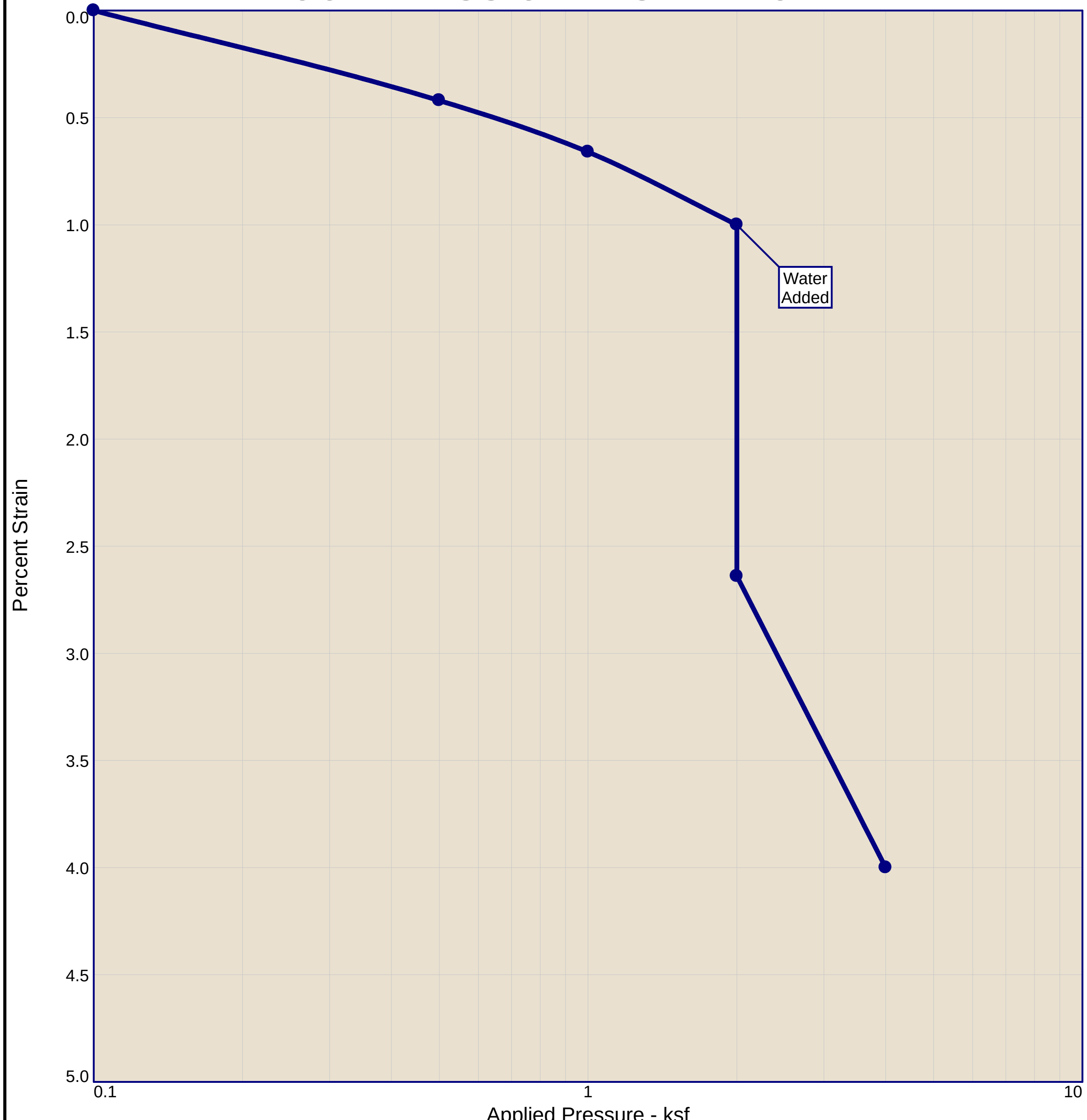
Remarks:

Western Technologies, Inc.

Flagstaff, AZ

Plate B-7

COMPRESSION TEST REPORT



Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (ksf)	P_c (ksf)	C_c	C_r	Swell Press. (ksf)	Clpse. %	e_o
Sat.	Moist.											
33.2 %	6.8 %	107.3			2.65						1.6	0.541

MATERIAL DESCRIPTION	USCS	AASHTO
SANDY, SILTY CLAY	CL-ML	

Project No. 2523JB119 **Client:** MR. CURT BANEY

Project: THE VILLAGE AT SADDLEROCK CROSSING

Source: RING SAMPLE **Depth:** 5-6 FEET **Sample No.:** BORING 9

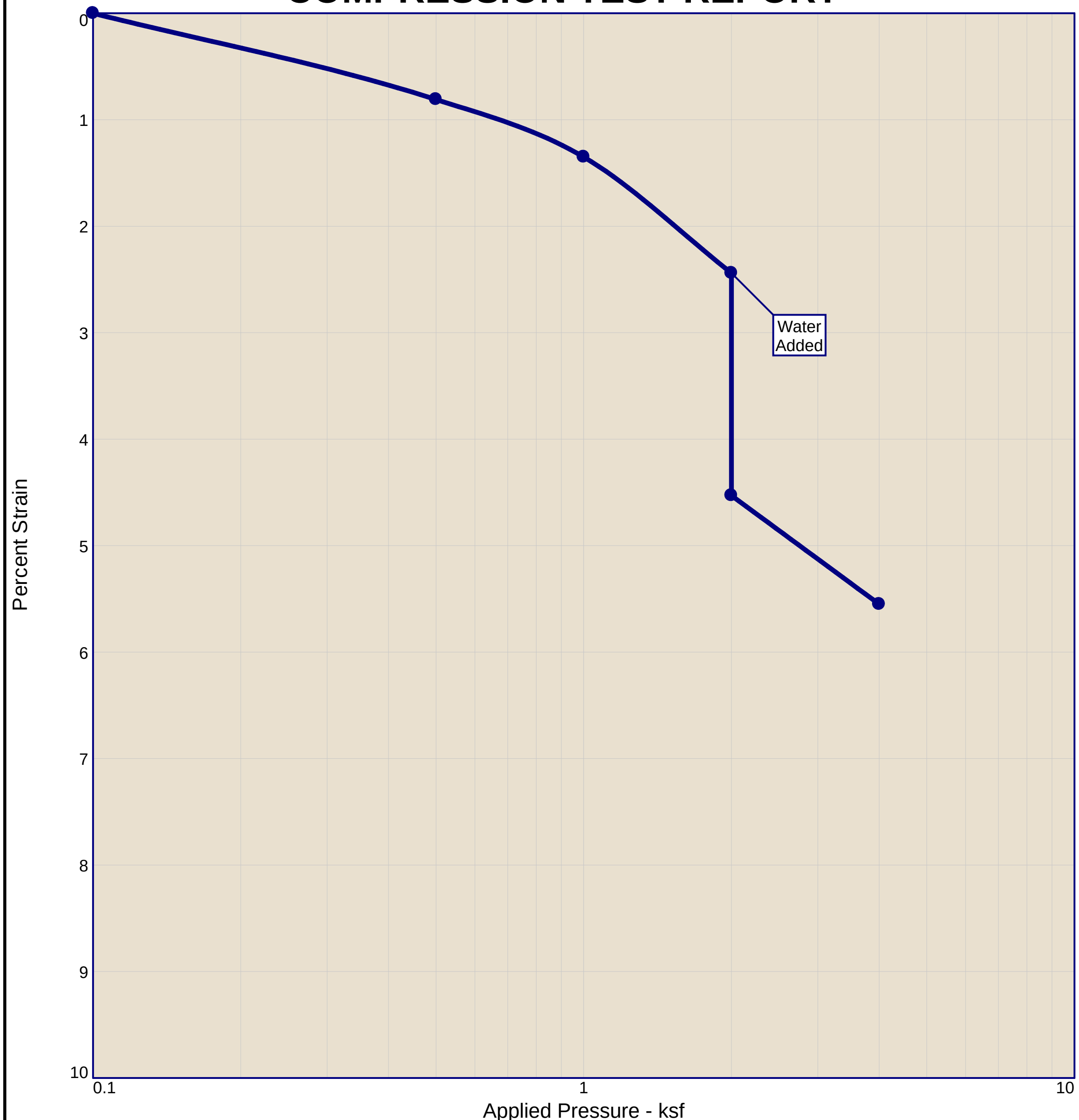
Remarks:

Western Technologies, Inc.

Flagstaff, AZ

Plate B-8

COMPRESSION TEST REPORT



Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (ksf)	P_c (ksf)	C_c	C_r	Swell Press. (ksf)	Clpse. %	e_o
Sat.	Moist.											
33.3 %	5.8 %	113.3			2.65						2.1	0.460

MATERIAL DESCRIPTION	USCS	AASHTO
SANDY, SILTY CLAY	CL-ML	

Project No. 2523JB119 **Client:** MR. CURT BANEY

Project: THE VILLAGE AT SADDLEROCK CROSSING

Source: RING SAMPLE **Depth:** 2-3 FEET **Sample No.:** BORING 12

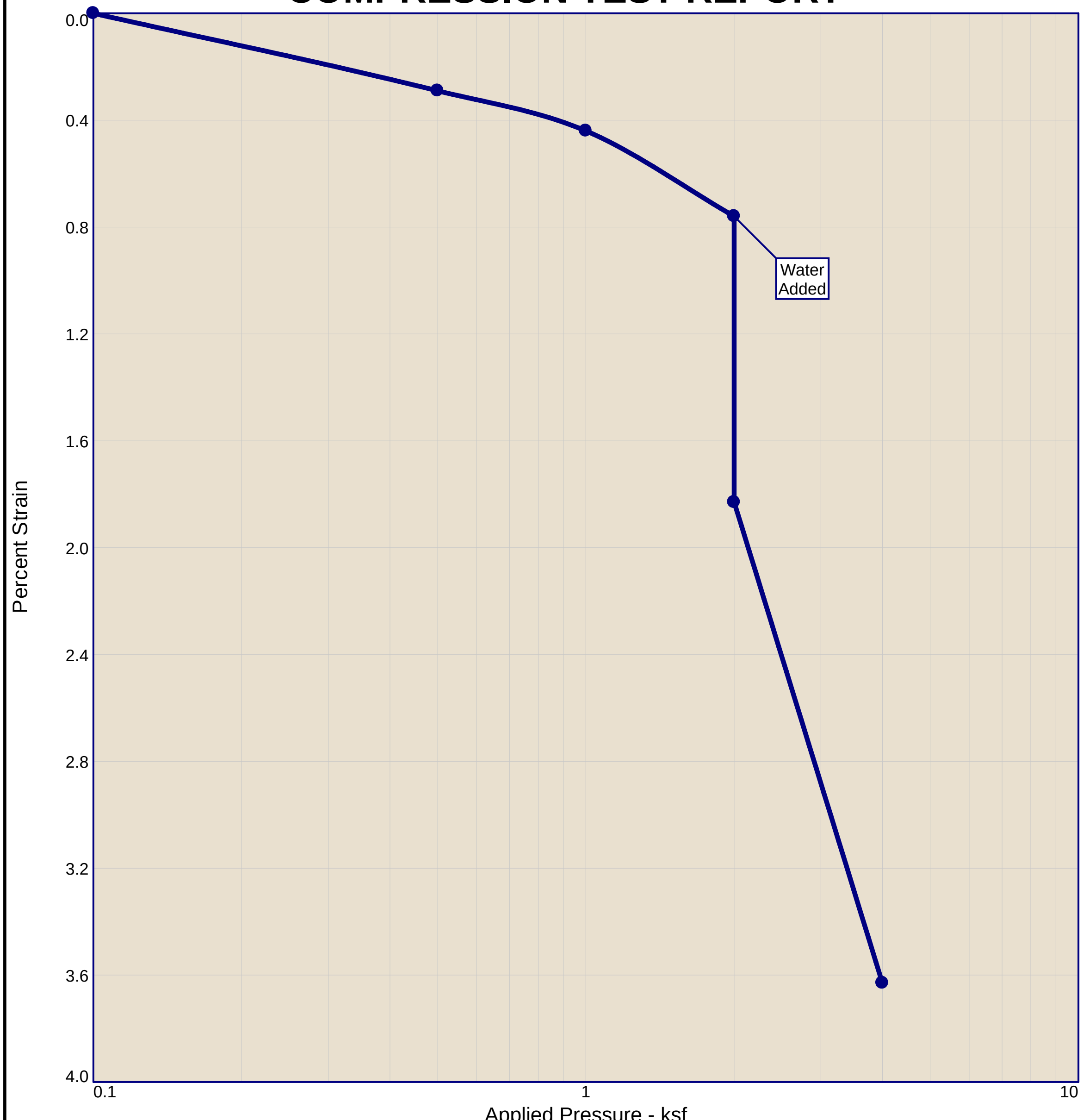
Remarks:

Western Technologies, Inc.

Flagstaff, AZ

Plate B-9

COMPRESSION TEST REPORT



Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (ksf)	P_c (ksf)	C_c	C_r	Swell Press. (ksf)	Clpse. %	e_o
Sat.	Moist.											
65.7 %	15.4 %	102.0			2.65						1.1	0.623

MATERIAL DESCRIPTION	USCS	AASHTO
SILTY, CLAYEY GRAVEL WITH SAND	GC-GM	

Project No. 2523JB119 **Client:** MR. CURT BANEY

Project: THE VILLAGE AT SADDLEROCK CROSSING

Source: RING SAMPLE **Depth:** 5-6 FEET **Sample No.:** BORING 13

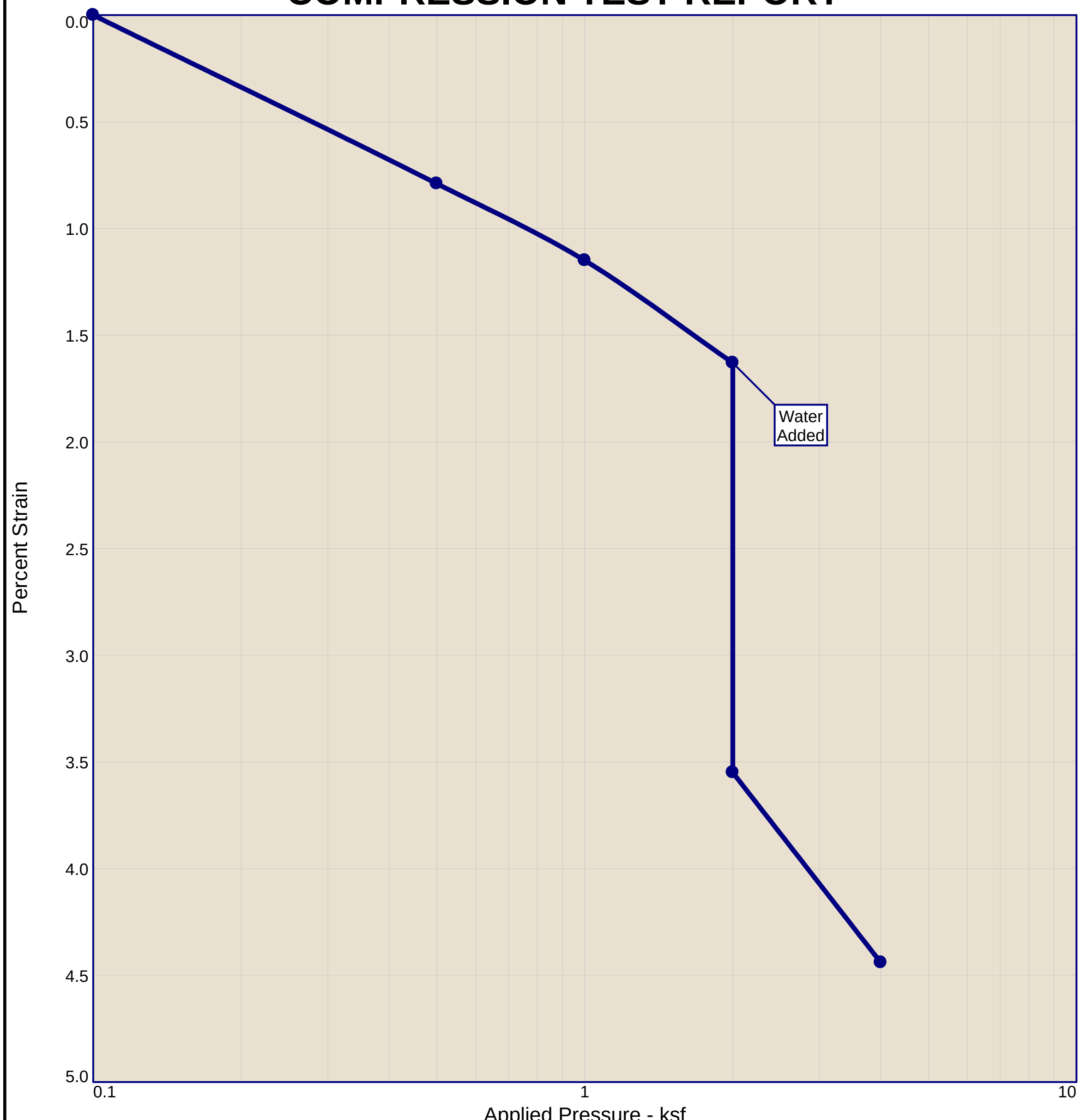
Remarks:

Western Technologies, Inc.

Flagstaff, AZ

Plate B-10

COMPRESSION TEST REPORT



Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (ksf)	P_c (ksf)	C_c	C_r	Swell Press. (ksf)	Clpse. %	e_o
Sat.	Moist.											
68.9 %	13.7 %	108.4			2.65						1.9	0.526

MATERIAL DESCRIPTION	USCS	AASHTO
SANDY, SILTY CLAY	CL-ML	

Project No. 2523JB119 **Client:** MR. CURT BANEY

Project: THE VILLAGE AT SADDLEROCK CROSSING

Source: RING SAMPLE **Depth:** 2-3 FEET **Sample No.:** BORING 17

Remarks:

Western Technologies, Inc.

Flagstaff, AZ

Plate B-11