

COMPREHENSIVE DESIGN SUBMITTAL

THE VILLAGE AT SADDLEROCK CROSSING Sedona, AZ



6.36	ACRE	ES
70,000	MAX	LUMENS PER ACRE
445,200	TOTA	L LUMENS ALLOWED

		-		
FULLY	2359	11,795		
FULLY	275	13,750		
	LUMEN TOTAL	395,212		
TOTAL LUM	ENS ALLOWED	445,200		

NO. OF LAMPS	QUANTITY	WATTS PER LAMP	Lamp Type	LIGHTING Plan Symbol	Shielding	Lumens per Fixture	Lumens Total
1	18	55	LED	А	FULLY	6484	116,712
1	1	55	LED	A1	FULLY	4838	4,838
1	8	55	LED	A1G	FULLY	4838	38,704
1	3	55	LED	AG	FULLY	6484	19,452
1	160	12	LED	В	FULLY	550	88,000
1	91	9	LED	С	FULLY	921	83,811
1	33	12	LED	D	FULLY	550	18,150
1	5	16	LED	E	FULLY	2359	11,795
1	50	7	LED	Н	FULLY	275	13,750



PHOTOMETRIC NOTES

REVIEWED BY THE ENGINEER COMPLIANCE WITH APPLICABLE LIGHTING CODES. THE CALCULATIONS ARE PERFORMED BY LIGHTING SUPPLIER AND THE ENGINEER'S REVIEW IS NOT A JARANTEE OF PERFORMANCE. SUBSTITUTIONS TO ANY TIXTURE WILL REQUIRE OMETRICS WHICH WILL THE FIXTURE SCHEDULE SHOWN ON THIS SHEET IS FOR ALCULATION PURPOSES ONLY. DO NOT USE AS A BILL-OF-MATERIAL. REFER TO HE PROJECT FIXTURE SCHEDUL

Sche	edule			
	Label	QTY	Manufacturer	Catalog Number
	A	18	VISIONAIRE	ODN-1-L-T5WR-32LC-5-27K-UNV-VA110-S1-3 1/2(FINISH)-C1/H6 / RSA 9.5' POLE W/ 3-1/2" O.D. X 4" TALL TENON TOP AND 2.5 BASE
	A1	1	VISIONAIRE	ODN-1-L-T1-32LC-5-27K-UNV-VA110-S1-3 1/2(FINISH)-C1/H6-CLS / RSA 5.5' POLE W/ 3-1/2" O.D. X 4" TALL TENON TOP AND 2.5 BASE
	A1G	8	VISIONAIRE	ODN-1-L-T1-32LC-5-27K-UNV-VA110-S1-3 1/2(FINISH)-C1/H6-CLS / RSA 5.5' POLE W/ 3-1/2" O.D. X 4" TALL TENON TOP AND 2.5 BASE
	AG	3	VISIONAIRE	ODN-1-L-T5WR-32LC-5-27K-UNV-VA110-S1-3 1/2(FINISH)-C1/H6 / RSA 5.5' POLE W/ 3-1/2" O.D. X 4" TALL TENON TOP AND 2.5 BASE
	В	160	ANP	WDU514-(FINISH)-M012LD-D-N-27K-E33UE14
	С	91	GOTHAM	EVO4 27/10 AR MD LSS
	D	33	ANP	WDU516-(FINISH)-M012LD-D-N-27K-E33UE18
	E	5	ANP	BL7803 FR M016LD W 27K (FINISH)
	Н	50	WAC Lighting	6061-2700

Description	Lamp	Filename	Lumens per Lamp	LLF	Wattage
20" DIA SMALL LED BELL HOUSING WITH T5 WIDE ROUND OPTIC, MOUNTED @12' AFG	2700K LED / 6484 LUMENS	ODN-1-L_T5WR_32LC _5_2700K.ies	6484	0.91	55
20" DIA SMALL LED BELL HOUSING WITH TYPE 1 OPTIC & HOUSE SIDE SHIELD, MOUNTED @12' AFG	2700K LED / 4838 LUMENS	ODN-1-L_T1_32LC_5 _2700K_UNV_CLS.ies	4838	0.91	54.7
20" DIA SMALL LED BELL HOUSING WITH TYPE 1 OPTIC & HOUSE SIDE SHIELD MOUNTED @8' AFG	2700K LED / 4838 LUMENS	ODN-1-L_T1_32LC_5 _2700K_UNV_CLS.ies	4838	0.91	54.7
20" DIA SMALL LED BELL HOUSING WITH T5 WIDE ROUND OPTIC, MOUNTED @8' AFG	2700K LED / 6484 LUMENS	ODN-1-L_T5WR_32LC _5_2700K.ies	6484	0.91	55
14" VINTAGE LAMP SHADE WITH INTEGRATED 12W CREE LED MODULE RATED FOR 60,000 HOURS	2700K LED / 550 LUMENS	LSU12M012LDDN40K.ies	550	0.91	11.73
EVO 4IN ROUND DOWNLIGHT, 80 CRI, 2700K, 1000LM, MED DIST, CLEAR SEMI-SPEC	2700K LED / 921 LUMENS	EVO4_27_10_AR_MD _LSS.ies	921	0.91	8.8
16" VINTAGE LAMP SHADE 12W CREE LED Module - Dome Lens	2700K LED / 550 LUMENS	LSU12M012LDDN40K.ies	550	0.91	11.73
BOLLARD	2700K LED / 2359 LUMENS		2359	0.5	16
LOCATIONS TBD	3000K LED / 275 LUMENS		275	1	6.5



Stephen Thompson Architect. Sedona/Del Mar Studio@StephenThompsonArchitect.com C: 928.301.5922



75 Kallof Place Sedona, AZ 86336

928.282.1061 928.282.72058 fax

www.swiaz.com

Engineering an environment of excellence

PRELIMINARY DRAINAGE REPORT for THE VILLAGE AT SADDLEROCK CROSSING

APNs: 408-26-004B, 408-26-004C, 408-26-009A, 408-26-009C, 408-26-010, 408-26-011, 408-26-012, 408-26-013, 408-26-014, 408-26-086A and 408-26-088 Sedona, Arizona

> Prepared for: Baney Corporation 475 NE Bellevue Dr. Suite 210 Bend, OR 97701

> > Job # 16034

TABLE OF CONTENTS

Introduction	1
Objective	1
Procedure	2
Results	3
Conclusion	3
References	3

APPENDIX

Vicinity Map FEMA FIRM Panel City of Sedona Flood Plain Management Study City of Sedona Storm Water Master Plan Pre-Developed Drainage Exhibit NOAA Atlas 14 Point Precipitation Frequency Estimate Table Grading and Utilities Concept Plan

Introduction

The proposed project site is located in Sedona, Arizona, south of State Route (SR)-89A, and between Saddlerock Circle and Elk Road. The project site is located on approximately 6.36 acres of vacant land, positioned in a portion of sections 12 & 13, Township 17 North, Range 5 East Gila and Salt River Meridian. A vicinity map is in the Appendix.

The proposed project consists of 4 hotel units, a lobby/restaurant building, 4 multifamily housing units, a parking structure, a parking lot, and the paving of a connector road from Saddlerock Circle to the intersection at Soldiers Pass Road and SR-89A. The proposed project is located on parcels 408-26-004B, 408-26-004C, 408-26-009A, 408-26-009C, 408-26-010, 408-26-011, 408-26-012, 408-26-013, 408-26-014, 408-26-086A and 408-26-088. The existing site has 70+ trees and shrubs, and one concrete driveway entrance along SR-89A on the north side of the site. An existing ditch on the west side of the property collects on-site and off-site runoff from the north, south, and east. The ditch has a 36" culvert which takes runoff west under Saddlerock Circle. Surrounding developments include the Saddlerock Homes subdivision to the west and south of the project site, commercial property to the west, north and east, and the Sedona Elks Lodge to the east.

The project is located in Zone X of the FEMA Flood Insurance Rate Map, map number 04025C1435G, effective September 3, 2010. Zone X is described as an area determined to be outside the 500-year floodplain. The preliminary FEMA Flood Insurance Rate Map number 04025C1435H, dated June 30, 2020, shows no changes to the flood hazard area designation for the site. The Appendix contains a portion of the FIRM near the project area.

The City of Sedona Flood Plain Management Study places part of the proposed project site within the 100-year floodplain boundary. This study also places the site in basin number 77, with a flow of 134 cfs for the 100-year storm event. The City of Sedona Storm Water Master Plan places the site in basin B77B, with a flow of 256 cfs for the 100-year event. Information from these studies can be found in the Appendix.

Objective

The objective of this report is to ascertain the impact the proposed development will have on the runoff characteristics of the site and to determine the detention volume needed to attenuate the additional post-development flows. The design of the proposed drainage control structures will be in accordance with City of Sedona and Yavapai County drainage criteria.

Procedure

The total project watershed is approximately 44.98 acres and is a mixture of developed residential housing, undeveloped native land, and previously graded vacant land. The project site slopes from east to west towards Saddlerock Circle, with an average slope of 5%. Off-site runoff from commercial property to the east drains west through the site. A catch basin on Saddlerock Circle collects runoff from SR-89A and flows through an 18" pipe to the ditch on the west side of the project site. Off-site flows from the neighborhood to the south of the site are routed through a network of ditches and culverts, which enters the ditch on the southwest corner of the site. Flow also enters the site from the cul-de-sac (end of Saddlerock Lane) and from the two properties to the east of Saddlerock Lane. When the pipe on Saddlerock Lane is full, runoff overtops the ditch on the east of Saddlerock Lane and enters the site at the cul-de-sac. On-site topographic survey was performed by Shephard Wesnitzer, Inc. in September 2018. Off-site topographic information was used from the 2007 City of Sedona Aerial Survey. The pre-development drainage map is provided in the Appendix.

The development of the project site includes the addition of approximately 3.66 acres of impervious surfaces. The resulting storm water runoff is proposed to be routed through a storm drain system from the east side of the project site across the proposed development to the west, where it then outlets into the existing 36" culvert under Saddlerock Circle. The proposed rainwater harvesting system, consisting of tanks collecting storm water from the hotel unit roofs, could potentially offset a portion of the direct storm runoff from the site, if approved. To mitigate increased peak flows from the development of the project site, an underground detention structure is proposed.

The design rainfall data was taken from the site specific NOAA Atlas 14 point precipitation frequency estimates table, as shown in the Appendix. The required storage volume of storm water runoff from the development of the site was determined based on retaining the storm runoff volume for the entire 100-year, 2-hour storm event from all added impervious areas of the project site, per the Yavapai County Drainage Manual.

Off-site flows from the northern portion of Saddlerock Circle and SR-89A will be collected through a catch basin and conveyed to the 36" culvert under Saddlerock Circle via a storm drain pipe. Off-site flows from the east along Elk Road will be collected through catch basins and directed under the proposed parking structure into the proposed storm drain system. Off-site flows from the south will also be conveyed to the proposed storm drain system through storm drain inlets. The development of the Village at Saddlerock Crossing project will not alter the existing off-site flowrate conditions with the proposed detention system.

<u>Results</u>

The underground detention structure is proposed to be located within the parking lot on the west side of the site, and will require approximately 36,200 ft.³ of volume to attenuate peak flows to pre-development rates. This volume can be attained through the placement of 1,280 ft. of 6' corrugated metal pipe beneath the parking lot. The first flush volume of approximately 6700 ft.³ will be retained below the basin outlet, with the excess storm water runoff being conveyed to the 36" culvert under Saddlerock Circle. Refer to the Grading and Utilities Concept Plan for preliminary details, grades, finished elevations, and locations.

Conclusion

A runoff volume for the 100-year, 2 hour storm event was calculated for the project watershed to determine a required detention volume of 36,200 ft³. Runoff from the development of the site, along with the off-site flows to the west, east, and south will be conveyed into the proposed underground detention basin through a storm drain system. The underground detention structure will discharge to the west through the existing 36" culvert underneath Saddlerock Circle.

The design concepts in this report will ensure that the drainage integrity of the site is sustained with proper maintenance activity. Activities include frequent clearing of debris and sediment from the storm drain inlets and detention areas, disturbed slope treatment and erosion control. Frequent monitoring will ensure expedient remedies to common problems such as erosion, sedimentation, and flow obstructions.

References

City of Sedona Flood Plain Management Study, City of Sedona, 1994

City of Sedona Storm Water Master Plan, City of Sedona, 2005

<u>Yavapai County Drainage Design Manual</u>, Yavapai County Flood Control District, 2015

Shephard-Wesnitzer, Inc. Consulting Civil Engineers Sedona, Arizona

APPENDIX



CHECKED JTL

PLOTTED: Apr 08, 2021-3:05pm

P: \2016 \16034 \ENGINEERING \DRAINAGE \DWG \16034 - VICINITY MAP.DWG EMETZ FILE:

National Flood Hazard Layer FIRMette



Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT



OD HAZARD

AIN IN

erestination rison 2,000

legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for

regulatory purposes.

34°51'28.1'

1,000

500

250

W"EB.01'74°111

34°51'57.64"N





CITY OF SEDONA FLOODPLAIN MANAGEMENT STUDY, 1994

NO SCALE

PORTION OF OVERALL DRAINAGE BASIN MAP

- NORTH NORTH



CITY OF SEDONA FLOODPLAIN MANAGEMENT STUDY, 1994

NO SCALE



FLOODPLAIN MANAGEMENT STUDY, 1994

NO SCALE

SDHEC236.DAT - Notepad File Edit Format View Help GR4410.0 0.0 4408.0 30.0 4406.3 80.0 4406.0 120.0 4405.0 150.0 GR4401.6 153.0 4401.6 158.3 4405.0 164.0 4406.0 210.0 4408.0 290.0 GR4410.0 350.0 NH 3 0.075 150.0 0.035 164.0 0.075 350.0 X13603.911150164101010GR4410.00.04408.030.04406.380.04406.0120.04405.0150.0GR4401.6154.04401.6157.04405.0164.04406.0210.04408.0290.0 GR4410.0 350.0
 NC
 0.1
 0.3

 QT
 7.
 7.
 23.
 39.
 71.
 101.
 134.
 198.

 NH
 3
 .075
 120.0
 .035
 134.0
 .075
 380.0

 X13604.1
 9
 120.0
 134.0
 355
 385
 366
 -0.5

 GR4418.0
 0.0
 4416.0
 120.0
 4411.4
 126.0
 4411.4
 129.0
 4416.0
 134.0
 GR4415.0 230.0 4415.0 330.0 4416.0 350.0 4418.0 380.0 * CROSS SECTION 3604.2 AT DOWNSTREAM CULVERT FACE- FLOW LIMITED TO CULVERT WIDTH * CROSS SECTION 3604.2 AT DOMINITURE 1 0.1 NC 0.3 0.5 NH 3 .075 120.0 .035 134.0 .075 380.0 X13604.2 9 126.0 129.0 20 20 20 4414.2 4414.2 GR4418.0 0.0 4416.0 120.0 4411.4 126.0 4411.4 129.0 4416.0 134.0 GR4415.0 230.0 4415.0 330.0 4416.0 350.0 4418.0 380.0 SC 1.024 0.50 2.8 3.0 50. 2.1 4412.4 4411.4 * CROSS SECTION 3605.1 AT UPSTREAM CULVERT FACE - FLOW LIMITED TO CULVERT WIDTH

 NH
 3
 0.075
 115.
 0.035
 126.
 0.075
 360.

 X13605.1
 8
 119.
 122.
 50.
 50.
 50.

 X2
 2
 4415.2
 4415.2
 4415.2

 BT
 -8
 1000.9
 4417.0
 1038.3
 4416.5
 1072.5
 4415.7

 BT
 1110.8
 4415.2
 1145.9
 4415.2
 1186.2
 4415.5

 BT
 1223.3
 4415.7
 1242.6
 4416.1
 6416.1
 684418.0

 GR4418.0
 0.0
 4416.0
 70.0
 4415.7
 115.0
 4412.4
 119.0
 4412.4
 122.0

GR4415.5 126.0 4416.0 320.0 4418.0 360.0 NH 3 0.075 115. 0.035 126. 0.075 360. X13605.2 8 115. 126. 10. 10. 10. GR4418.0 0.0 4416.0 70.0 4415.7 115.0 4412.4 119.0 4412.4 122.0 GR4415.5 126.0 4416.0 320.0 4418.0 360.0 0.1 0.3 NC NH 1 0.055 1261.8 X13606.0 8 1184.6 1220.4 345 315 352 GR4426.6 1000.0 4424.6 1061.0 4423.8 1103.7 4423.2 1141.5 4422.7 1184.6 GR4423.0 1220.4 4426.5 1259.6 4426.6 1261.8 EJ







NOAA Atlas 14, Volume 1, Version 5 Location name: Sedona, Arizona, USA* Latitude: 34.8621°, Longitude: -111.7837° Elevation: 4438.28 ft** source: ESRI Maps

** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹										
Duration	Average recurrence interval (years)									
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	0.211 (0.176-0.252)	0.272 (0.227-0.325)	0.366 (0.304-0.437)	0.445 (0.371-0.530)	0.558 (0.461-0.661)	0.652 (0.534-0.773)	0.753 (0.612-0.894)	0.862 (0.691-1.03)	1.02 (0.804-1.22)	1.16 (0.897-1.39)
10-min	0.321 (0.269-0.383)	0.414 (0.345-0.494)	0.557 (0.463-0.665)	0.678 (0.564-0.807)	0.849 (0.701-1.01)	0.992 (0.813-1.18)	1.15 (0.932-1.36)	1.31 (1.05-1.56)	1.56 (1.22-1.86)	1.76 (1.37-2.12)
15-min	0.398 (0.333-0.475)	0.513 (0.428-0.612)	0.691 (0.574-0.824)	0.840 (0.699-1.00)	1.05 (0.869-1.25)	1.23 (1.01-1.46)	1.42 (1.16-1.69)	1.63 (1.30-1.94)	1.93 (1.52-2.31)	2.18 (1.69-2.63)
30-min	0.537 (0.448-0.639)	0.691 (0.575-0.824)	0.930 (0.773-1.11)	1.13 (0.942-1.35)	1.42 (1.17-1.68)	1.66 (1.36-1.96)	1.91 (1.56-2.27)	2.19 (1.76-2.61)	2.60 (2.04-3.11)	2.94 (2.28-3.54)
60-min	0.664 (0.554-0.791)	0.855 (0.712-1.02)	1.15 (0.957-1.37)	1.40 (1.17-1.67)	1.75 (1.45-2.08)	2.05 (1.68-2.43)	2.37 (1.93-2.81)	2.71 (2.17-3.23)	3.21 (2.53-3.85)	3.63 (2.82-4.38)
2-hr	0.782 (0.680-0.910)	0.990 (0.853-1.16)	1.31 (1.13-1.52)	1.58 (1.35-1.83)	1.96 (1.67-2.27)	2.28 (1.92-2.65)	2.63 (2.19-3.07)	3.02 (2.47-3.51)	3.58 (2.88-4.17)	4.04 (3.20-4.72)
3-hr	0.840 (0.738-0.972)	1.06 (0.937-1.23)	1.36 (1.19-1.57)	1.62 (1.41-1.87)	1.99 (1.72-2.30)	2.31 (1.98-2.66)	2.66 (2.25-3.08)	3.05 (2.54-3.52)	3.61 (2.96-4.20)	4.07 (3.27-4.77)
6-hr	1.02 (0.913-1.13)	1.27 (1.14-1.41)	1.57 (1.41-1.75)	1.85 (1.65-2.05)	2.24 (1.98-2.49)	2.55 (2.24-2.84)	2.90 (2.52-3.23)	3.26 (2.80-3.65)	3.80 (3.21-4.29)	4.24 (3.52-4.82)
12-hr	1.31 (1.18-1.45)	1.62 (1.47-1.80)	1.98 (1.78-2.19)	2.28 (2.04-2.51)	2.68 (2.40-2.96)	3.00 (2.67-3.30)	3.33 (2.93-3.68)	3.66 (3.19-4.05)	4.13 (3.55-4.60)	4.51 (3.84-5.05)
24-hr	1.65 (1.50-1.81)	2.05 (1.87-2.27)	2.55 (2.32-2.82)	2.96 (2.68-3.27)	3.52 (3.17-3.88)	3.96 (3.56-4.36)	4.41 (3.94-4.87)	4.88 (4.34-5.39)	5.51 (4.86-6.12)	6.01 (5.25-6.70)
2-day	1.92 (1.75-2.12)	2.39 (2.17-2.64)	2.97 (2.71-3.28)	3.44 (3.12-3.79)	4.08 (3.70-4.50)	4.59 (4.14-5.05)	5.12 (4.59-5.63)	5.66 (5.04-6.25)	6.41 (5.64-7.09)	6.98 (6.10-7.75)
3-day	2.06 (1.88-2.27)	2.57 (2.34-2.83)	3.20 (2.92-3.53)	3.72 (3.38-4.09)	4.44 (4.02-4.88)	5.01 (4.51-5.50)	5.60 (5.02-6.15)	6.22 (5.53-6.85)	7.07 (6.23-7.81)	7.74 (6.77-8.59)
4-day	2.21 (2.02-2.42)	2.75 (2.51-3.03)	3.44 (3.14-3.78)	4.00 (3.64-4.40)	4.79 (4.35-5.25)	5.42 (4.89-5.94)	6.09 (5.46-6.68)	6.77 (6.03-7.44)	7.74 (6.82-8.54)	8.50 (7.43-9.43)
7-day	2.59 (2.37-2.83)	3.22 (2.95-3.53)	3.99 (3.65-4.36)	4.62 (4.23-5.06)	5.50 (5.02-6.02)	6.20 (5.64-6.79)	6.93 (6.27-7.60)	7.69 (6.91-8.43)	8.73 (7.77-9.62)	9.55 (8.42-10.5)
10-day	2.95 (2.70-3.23)	3.66 (3.35-4.02)	4.51 (4.13-4.94)	5.18 (4.74-5.68)	6.09 (5.54-6.65)	6.79 (6.16-7.43)	7.50 (6.76-8.22)	8.21 (7.37-9.01)	9.17 (8.17-10.1)	9.90 (8.77-10.9)
20-day	3.81 (3.50-4.16)	4.72 (4.34-5.16)	5.72 (5.27-6.26)	6.48 (5.95-7.07)	7.45 (6.83-8.13)	8.17 (7.46-8.91)	8.87 (8.08-9.69)	9.54 (8.66-10.4)	10.4 (9.38-11.4)	11.0 (9.89-12.1)
30-day	4.58 (4.20-5.00)	5.68 (5.21-6.19)	6.85 (6.27-7.47)	7.74 (7.08-8.42)	8.87 (8.09-9.64)	9.68 (8.81-10.5)	10.5 (9.49-11.4)	11.2 (10.1-12.3)	12.2 (11.0-13.3)	12.8 (11.5-14.1)
45-day	5.41 (4.94-5.97)	6.72 (6.13-7.41)	8.12 (7.41-8.94)	9.19 (8.38-10.1)	10.6 (9.62-11.6)	11.6 (10.5-12.7)	12.6 (11.4-13.8)	13.5 (12.2-14.9)	14.7 (13.2-16.2)	15.6 (13.9-17.2)
60-day	6.31 (5.75-6.91)	7.82 (7.14-8.57)	9.40 (8.57-10.3)	10.6 (9.62-11.6)	12.0 (10.9-13.2)	13.1 (11.9-14.3)	14.1 (12.7-15.4)	15.0 (13.6-16.5)	16.2 (14.5-17.7)	17.0 (15.2-18.7)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

Back to Top

PF graphical



NOAA Atlas 14, Volume 1, Version 5

Created (GMT): Thu Feb 4 21:19:51 2021

Back to Top

Maps & aerials



Large scale terrain





Precipitation Frequency Data Server





Back to Top

US Department of Commerce National Oceanic and Atmospheric Administration National Weather Service National Water Center 1325 East West Highway Silver Spring, MD 20910 Questions?: <u>HDSC.Questions@noaa.gov</u>

Disclaimer

